FLOOD CHARACTERISTICS OF MISSISSIPPI STREAMS

U.S. GEOLOGICAL SURVEY Water-Resources Investigations Report 91-4037



Prepared in cooperation with the MISSISSIPPI STATE HIGHWAY DEPARTMENT

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By Mark N. Landers and K. Van Wilson, Jr.

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Jackson, Mississippi 1991

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CONVERSION FACTORS AND VERTICAL DATUM

Multiply	<u>By</u>	To obtain
inch (in.)	2.54	centimeter
foot (ft)	0.3048	meter
mile (mi)	1.609	kilometer
foot per mile (ft/mi)	0.018939	meter per kilometer
square mile (mi²)	2.590	square kilometer
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second

Sea Level: In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929 (NGVD of 1929)--a datum derived from a general adjustment of the first-order level nets of both the United States and Canada, formerly called Sea Level Datum of 1929.

FLOOD CHARACTERISTICS OF MISSISSIPPI STREAMS

by Mark N. Landers and K. Van Wilson, Jr.

ABSTRACT

Flood magnitudes for selected recurrence intervals from 2 to 500 years were determined for 330 gaged sites in the study area where annual peak-flow records have been collected. The principal study area is Mississippi; however, selected data collected in adjoining States on streams draining into or from Mississippi are also included. Flood frequency at a gaged stream site is defined by fitting the Pearson Type III probability distribution to the log-transformed annual peaks. The accuracy of the flood frequency determined for a gaged site is determined primarily by the number of years of annual peak-flow record (the sample size). Greater accuracy is achieved in the current analysis than in previous analyses because of the additional years of annual peak-flow record. Flood-frequency and basin characteristics at gaged sites were used to develop regression equations for estimating flood frequency where annual peak-flow records are not available.

Flood frequency for ungaged stream sites in Mississippi may be estimated using basin characteristics in regression equations. Regression equations were computed using the generalized-least-squares procedure rather than the ordinary-least-squares procedure used in previous regional hydrologic analyses. The generalized-least-squares procedure considers the variable error of the gaging station flood frequencies and corrects for the cross-correlation of concurrent annual peaks. When the gaging stations in the sample for regression analysis have widely varying record lengths and concurrent peak flows, which are correlated between sites, the generalized-least-squares procedure provides more accurate estimates of the regression coefficients and model error than does the ordinary-least-squares procedure. These flood-frequency equations provide managers with improved tools for estimating flood frequencies for purposes of management and design.

INTRODUCTION

The magnitude and frequency of floods are key factors in the design of bridges, highway embankments, culverts, levees, dams, and other structures near streams. Effective flood-plain management and the determination of flood insurance rates also require information on the magnitude and frequency of floods.

The Mississippi State Highway Department and the Federal Highway Administration recognize the need for adequate flood-frequency information for the safe, efficient design of drainage structures and roadways in Mississippi. Because of this need, the U.S. Geological Survey, in cooperation with the Mississippi State Highway Department, conducted a study to update previous flood-frequency reports using data collected through the 1988 water year. A water year, which is the 12-month period from October 1 to September 30, is designated by the calendar year in which it ends. Thus, the 12-month period ending September 30, 1988, is called the "1988 water year."

Purpose and Scope

The purpose of this report is to provide techniques for estimating the magnitude of floods with selected recurrence intervals from 2 to 500 years for streams in Mississippi. This report supersedes an earlier one by Colson and Hudson (1976) because of additional available data and new analytical techniques.

The principal study area is Mississippi; however, selected data collected in adjoining States on streams draining into or from Mississippi are also included (fig. 1). Estimates of flood magnitude are presented for 330 streamflow gaging stations in the study area. Regional estimating equations were developed to provide flood-frequency information at ungaged locations. The regional flood-frequency equations were developed using a new procedure, generalized-least-squares regression (Stedinger and Tasker, 1985, 1986), which better addresses statistical problems of hydrologic variables in regional hydrologic analyses than does ordinary-least-squares regression. Flood-frequency equations were developed for streams in four subgroups; three defined by geographic region and one by drainage-area magnitude. Additional equations for urban areas are presented from "Flood Characteristics of Urban Watersheds in the United States" (Sauer and others, 1983).

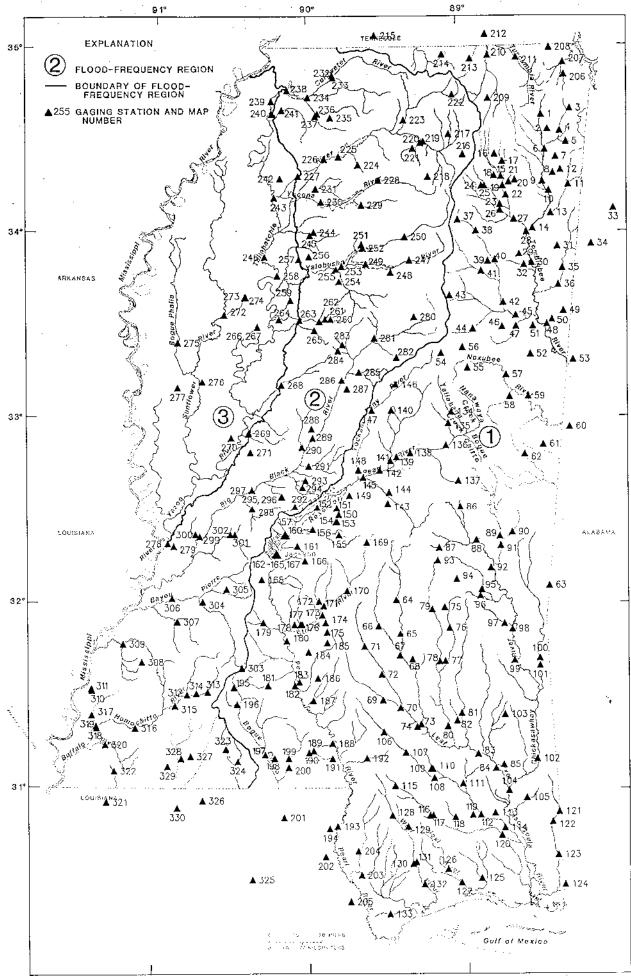


Figure 1.--Location of streamflow-gaging stations and flood-frequency regions.

General Description of Study Area

Mississippi is in the East Gulf Coastal Plain and includes parts of several physiographic districts, but the State generally may be divided into the coastal plain uplands and the lower Mississippi River Alluvial Plain, known locally as the "Delta." The transition between the coastal plain uplands and the Delta is an abrupt, dissected escarpment characterized by steep slopes and pronounced ridges rising 150 to 250 ft above the alluvial plain.

The Delta, in the northwestern part of the State, is a flat, lens-shaped basin having a maximum width of about 65 mi. The topography is a series of abandoned meander belts, oxbow lakes, and swamps. Regional drainage characteristics are broad, widely meandering stream courses trending to the southwest with low channel slopes and large amounts of depression and channel storage. Extensive levees protect all but the southern part of the alluvial plain from floodwaters of the Mississippi River.

The coastal plain uplands is composed of hilly uplands and gently undulating prairies. The maximum elevation in the State is located in the coastal plain uplands in the northeast corner of the State, where elevations reach about 806 ft above sea level.

The six major drainage basins in Mississippi are the Yazoo, Big Black, Homochitto, Tombigbee, Pascagoula, and Pearl. The Yazoo, Big Black, and Homochitto basins drain southwestward into the Mississippi River. The Tombigbee basin drains southward into the Mobile River. The Pascagoula and Pearl basins drain southward into the Gulf of Mexico.

The climate of Mississippi is controlled primarily by the proximity of the Gulf of Mexico and the prevailing southwesterly winds. These conditions contribute to a generally warm and humid climate, making Mississippi one of the two wettest States in the contiguous United States. The average annual precipitation ranges from 54 inches in the northern part of the State to about 60 inches in the southern part (Wax, 1982).

<u>Acknowledgments</u>

The U.S. Army Corps of Engineers and many State and local agencies are acknowledged for their cooperation in the collection of much of the data used in the study.

RECORDS OF FLOODING

Records of annual peak flow collected at streamflow-gaging stations provide the empirical basis for estimates of flood characteristics. In this study, the records were analyzed for 330 locations (fig. 1, table 1) on streams in and near Mississippi to provide a total of 8,470 years of systematic annual peakflow records.

Systematic records represent a random sample of annual peak flows at a site, generally collected over a continuous period. The distribution of systematic peak-flow-record lengths used in the regional analyses is shown in figure 2. A minimum of 10 years of record was considered necessary for estimating flood characteristics for a gaged site.

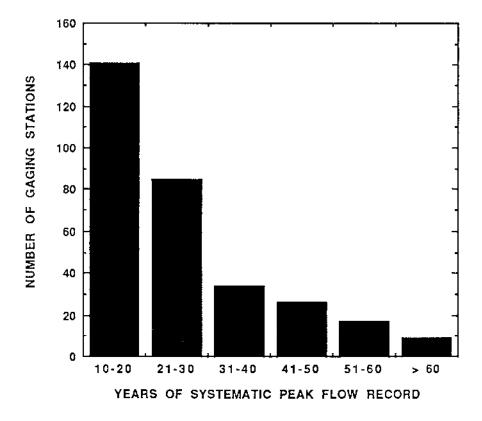


Figure 2.-- Distribution of systematic peak-flow record lengths.

Floods of unusually large magnitude often occur at a site when systematic records are not being obtained from a streamflow gage. In this study, evidence of the occurrence of unusually large floods was obtained from newspaper files, old records of stage, local historical records, diaries, and from individuals who remembered the flood or were informed by their ancestors. This flood information, referred to as historical record in this report, was used when available to extend the record of the largest floods at a site to a historical period much longer than that of the systematic record. Historical record is available for about 40 percent of the Mississippi sites having 10 or more years of systematic record.

Synthetic data (flood peaks generated from climatic records in a rainfall-runoff model) were used in the report by Colson and Hudson (1976) to extend the length of record at 89 gage sites, ranging in drainage area from 0.04 to 4.35 mi². Additional data have been collected since 1976, and 84 of these sites now have 10 or more years of systematic record. The synthetic and recorded flood-frequency discharges for these gages were compared using a paired Student's t-test, as described by Thomas (1987). The Student's t-tests, at the 5-percent level of significance for the 2-year to 100-year discharges, indicate that the synthetic data are statistically different from the systematic data for all except the 2-year and 5-year discharges. At the 1-percent level of significance, the difference was significant only at the 25-year and 100-year discharge. The bias of the synthetic data at the 5-percent level of significance was also reported by Colson (1986) and Thomas (1987). Therefore, the flood-frequency discharges based only on the systematic record were used in this study.

STATISTICAL CHARACTERISTICS OF ANNUAL PEAK FLOW

Statistical methods of analysis are well suited to the random nature of annual flooding. Statistical methods may be used to estimate flood frequency from a sample of recorded annual peak flows at a stream site using the assumption that the recorded sample represents the population of all the recorded and unrecorded annual peak flows. The Interagency Advisory Committee on Water Data (IACWD, 1982) recommends that the Pearson Type III distribution be used as the probability model for log-transformed annual peak-flow data. The Pearson Type III distribution requires estimates of the population mean, variance, and skew at a site. These population parameters

are estimated by computing the corresponding sample parameters, based on the systematic record, as follows:

$$\overline{X} = \frac{1}{N} \sum_{i=1}^{N} X_i \tag{1}$$

$$S^{2} = \frac{1}{N-1} \sum_{i=1}^{N} (X_{i} - \overline{X})^{2}$$
 (2)

$$G_{S} = \frac{N}{(N-1)(N-2)S^{3}} \sum_{i=1}^{N} (X_{i} - \overline{X})^{3}$$
(3)

where

 \overline{X} is the sample mean;

S is the sample standard deviation;

S² is the sample variance;

G_s is the sample skew;

 X_i is the log-transformed annual peak flow for year i; and

N is the sample size, that is, the number of years of peak-flow record for the stream site.

Previous studies of the sampling distribution of sample skew (G_s) have shown that G_s is a biased estimator of the population skew and is subject to large sampling variances as compared with \overline{X} and S^2 . Empirical bias correction factors were computed by Wallis and others (1974) based on Monte Carlo experiments. A bias correction equation based on record length and described by Tasker and Stedinger (1986) was used by Landers (1989), and is defined as:

$$C_b = (1 + 6/N)$$
 (4)

where C_b is the bias correction coefficient and N is as defined previously. Sample skew coefficients were multiplied by this bias correction coefficient and used to develop unbiased regional skew coefficients.

Tasker and Stedinger (1986) showed only minor differences between bias correction coefficients from this equation and from the empirical results of Wallis and others (1974), when N is greater than 20 and the absolute value of $G_{\rm S}$ is less than 1.0.

Population skew estimates are improved when computed from the weighted average of the sample and unbiased regional skew estimates for a site, as recommended by the IACWD (1982). Sample skew (G_s) is weighted inversely to its mean square error (MSE_s), and regional skew (G_r) is weighted inversely to an estimate of its sampling variance (MSE_r). The IACWD (1982) uses mean square error (MSE_r) as an estimate of the sampling variance of regional skew. Population skew then is estimated by:

$$\hat{G} = \frac{(MSE_r * G_S) + (MSE_S * G_r)}{MSE_r + MSE_S}$$
(5)

where

 \hat{G} is an estimate of the population skew coefficient, and G_r is assumed to be unbiased so that MSE_r is equal to the sampling variance of G_r .

Further improvements in estimated population skew are obtained by using weighted methods to estimate regional skew. Regional skew for Mississippi streams was studied in detail, and was described by Landers (1989) in a report that included a comparison of estimation techniques. The selected regional estimator is an unbiased, weighted-grid skew map. Regional skew coefficients for Mississippi are discussed in the Appendix.

FLOOD-FREQUENCY ESTIMATES FROM STREAMFLOW RECORDS

Flood-frequency estimates from records of annual peak flow at 330 gaging stations were computed by fitting the three-parameter Pearson Type III distribution to the sample of log-transformed annual peak flows, as recommended by the IACWD (1982). The regional unbiased map skew

developed by Landers (1989) was used with the biased station skew to provide the Water Resources Council (WRC) weighted estimation of population skew. (The existing IACWD guidelines do not recommend the unbiasing of station skew.) Computations were made using U.S. Geological Survey computer program J407, "Annual Flood Frequency Analysis Using WRC Guidelines" (Lepkin and others, 1981).

Stream basins were reviewed to determine if the basins were affected by regulation or channelization, which may violate the stationary time series assumption and make the station unrepresentative of regional flood-frequency characteristics. Data from sites in basins that were regulated or channelized during the period of record were analyzed to determine the effect of regulation or channelization (noted in table 1) on annual peak-flow records. In several basins, gages were in place prior to significant regulation or channelization. For each of these gaging stations, the period of record prior to significant regulation or channelization was used to expand the natural, regional data base. However, flood-frequency information for these gages given in table 1 represents existing conditions. Station flood-frequency values are not weighted with regional values for regulated or channelized streams.

Weighted Flood-Frequency Estimates

If two independent estimates of flood frequency are weighted in inverse proportion to their error (variance), the error of the weighted average is less than that of either estimate (IACWD, 1982). The regional flood-frequency estimates developed in this investigation are assumed to be independent of the station flood-frequency estimates. The two estimates were weighted inversely proportional to their respective time-sampling and prediction errors to obtain a best estimate of flood-frequency at each gage in accordance with Appendix 8 of Bulletin 17B (IACWD, 1982). The estimates shown in table 1 for selected recurrence intervals from 2 to 500 years are weighted estimates unless otherwise noted.

Ungaged Sites on Gaged Streams

Flood-frequency estimates from a gaged stream site may be extrapolated to an ungaged site on the same stream using drainage area ratios raised to the 0.6 power. This procedure is suggested if the drainage area at an ungaged site is within 50 percent of the drainage area at the gaged site on the same stream. This extrapolated estimate and the regional regression estimate for the ungaged site are weighted in the following equation:

$$Q_{T(w)} = 4\left(\frac{\Delta A}{A_g}\right)^2 Q_r + \left[1-4\left(\frac{\Delta A}{A_g}\right)^2\right] \left(\frac{A_u}{A_g}\right)^{0.6} Q_g$$
 (6)

where

Q_{T(w)} is the weighted discharge, in cubic feet per second, at the ungaged site for a recurrence interval of T years;

Qg is the weighted gage discharge, in cubic feet per second, for the selected recurrence interval, from table 1;

Qr is the regional regression discharge, in cubic feet per second, at the ungaged site for the selected recurrence interval;

 A_u is the drainage area, in square miles, at the ungaged site;

Ag is the drainage area, in square miles, at the gaged site; and

 ΔA is the difference between the drainage areas at the gaged and ungaged sites.

Where the drainage area at an ungaged site differs by more than 50 percent from that at the gaged site, the regional estimate should be used. If an ungaged site is between two gaged sites on the same stream, the suggested "50 percent rule" should be applied to determine which gaged site, if either, should be used to make an adjustment to the regional estimate at the

ungaged site. If the drainage area at the ungaged site is within 50 percent of that at both gaged sites, the flood-frequency estimate for the ungaged site can be interpolated logarithmically, on the basis of drainage area, between the weighted gage discharges (Q_g) from each gaged site.

Accuracy of Flood-Frequency Estimates for Gaged Stream Sites

"Streamflow characteristics can only be estimated; their true value can never be determined because there is a time-sampling error in every record of streamflow and a model error in every analytical method" (Hardison, 1969). It is important to evaluate the error associated with a given flood estimate because of the large range of accuracy that may be obtained in flood estimates using different methods. A measure of the accuracy or error of a flood estimate is necessary to evaluate the confidence or factor of safety with which it should be used, to compare and select methods of estimation, and to serve as a basis for risk analysis. Accuracy may be indicated by the variance or standard error of estimate.

Flood estimates from peak-flow records may contain errors due to: (1) any systematic measurement or computational errors, (2) use of an unrepresentative population probability distribution, or (3) errors in estimation of the population parameters defining the frequency distribution (time-sampling errors). The first source of error is addressed by quality assurance procedures in the data collection, computation, and review process. These errors generally are small and, in fact, non-systematic. The second source of error exists because the population of floods defies consistent, precise representation by any frequency distribution. The third source of errors lies in the estimation of population frequency distribution parameters for the sample data. This time-sampling error is assumed to be large, compared to the other two sources of error discussed. Time-sampling error is the only error quantified in the standard error of a flood-magnitude estimate from station peak-flow records for a recurrence interval (T). The standard error of the T-year flood estimate is the sum of errors in the estimation of the mean, the standard deviation, and the skew of the Pearson Type III distribution from the logarithms of annual peak flow for a given site. The time-sampling error is a function of the slope of the frequency curve (sample standard deviation), the estimated skewness, the recurrence interval (T) being

estimated, and the length of record as a measure of how representative the sample may be of the population of annual peaks. Methods of computing the standard (time-sampling) error have been presented by different authors [Bobee (1973), (Hardison (1971), and Kite (1988)]. This report uses the method described by Kite (1988) to compute the time-sampling errors for each station flood-frequency estimate. These time-sampling errors are combined with the error of prediction of the regional estimator to compute the standard error of the weighted estimate. The standard error, in percent, is shown in table 1 for the corresponding weighted or station flood-flow estimates.

The standard error of estimate is an indicator of the accuracy of a flood-frequency estimate. It is the square root of the variance of estimate about the unknown, true value being estimated. When errors are normally distributed, about two-thirds of the estimates are expected to lie within one standard error greater than or less than the true value. Ninety-five percent of the estimates are expected to be within two standard errors greater than or less than the true value. In this report, standard error is reported as a percentage of the true value being estimated. Thus, if a 10-year flood of magnitude 1,000 ft³/s has a standard error of 30 percent, the true value would be expected to be between about 700 and 1,300 ft³/s about two-thirds of the time.

Historical Record Evaluation

Evaluation of historical record in flood-frequency analyses is complex, and the most appropriate method is not certain at this time. In this investigation, historical records are included in the computation of Pearson Type III flood-frequency estimates using the adjusted-moment method recommended by IACWD (1982) and included in the J407 computer program (Lepkin and others, 1981). The effective record length obtained from the contribution of information from the historical record is required for computing the standard error of station flood-frequency estimates and for weighting station estimates with regional flood-frequency estimates. For a given recurrence interval, the effective record length is the number of years of systematic data that would produce the same standard error as a given combination of historical and systematic data (Stedinger and Cohn, 1986). Effective record length for stations having historical records was based on results of Monte Carlo simulations by Stedinger and Cohn (1986), which were

provided in a sub-routine of the generalized-least-squares regression model by Tasker and Stedinger (1989). The effective record length computed in the generalized-least-squares regression model was used in the computation of standard error.

Flood-Frequency of the Pearl River Main Stem

The Pearl River is formed by the confluence of Tallahaga Creek, Nanawaya Creek, and Bogue Chitto about 85 miles northeast of Jackson, Miss., and flows southward through Jackson and into the Gulf of Mexico (fig. 1). Upstream of Jackson, the drainage basin is average- or fan-shaped; whereas, the basin is typically elongated downstream, except where major tributaries flow into the Pearl River causing the basin shape to fan out.

The Ross Barnett Reservoir, located about 10 miles northeast of Jackson (fig. 1), is designed primarily for water supply and recreation; however, it has been used successfully to provide some flood-peak reduction. The reservoir normally is operated so that the pool is maintained at a near-constant level, and the inflow is passed through without significant attenuation. Since completion of the reservoir, the largest and third largest floods of record on the Pearl River at Jackson occurred in April 1979 and in May 1983. These flood-peak discharges at the Jackson gage, located at U.S. Highway 80 in south Jackson, were compared with discharges at State Highway 43 (Meeks Bridge) near Canton (about 12 miles upstream from the dam). The comparison indicated the May 1983 flood peak was reduced about 20 percent by the regulation of the Ross Barnett reservoir discharge, and the April 1979 flood peak may have been reduced a lesser amount. Some natural flood-peak attenuation between the gages at Meeks Bridge and at Jackson was expected due to variation in basin shape, which was indicated by adjusting the peak discharges at Meeks Bridge and at the Jackson gage for basin shape.

A basin shape coefficient developed by Wilson and Trotter (1961) was used in this report to demonstrate the effect of basin shape on flood flows along the Pearl River main stem (fig. 3). This basin shape coefficient is inversely related to the ratio of the distance that flood waters must travel divided by average width of the basin. Wilson and Trotter (1961) reported shape coefficients ranging from 0.55 to 1.48 for corresponding length-to-width ratios ranging from 13.0 to 1.50 for Mississippi streams (excluding the Delta).

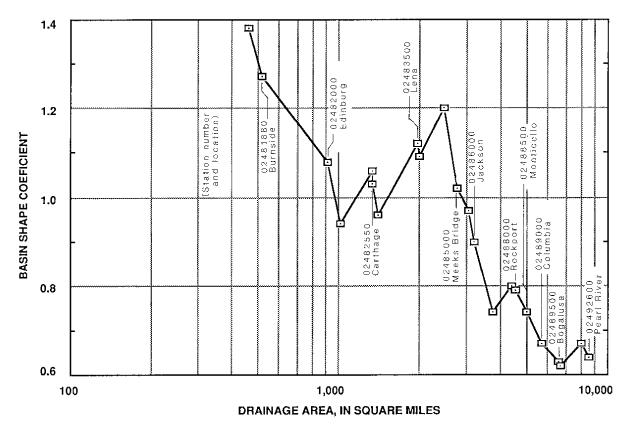


Figure 3.--Relation of basin shape coefficient to drainage area for the Pearl River main stem (method from: Wilson and Trotter, 1961).

The ratio is computed using $r = L^2/A$, where r is the ratio, L is the valley length, and A is the drainage area. For a drainage area of 1.0 mi², L for shape coefficients of 0.55 and 1.48 are 3.61 and 1.22 mi, respectively. Therefore, a larger basin shape coefficient would suggest a more fan-shaped basin; whereas, the smaller coefficient would suggest a more elongated basin with the coefficient of 1.0 for an average-shaped basin. The elongated basin would tend to provide more channel storage and dissipation of flood flows primarily because of the longer flow length.

The basin shape coefficient was determined along the Pearl River main stem from Burnside (near the confluence of Tallahaga Creek, Nanawaya Creek, and Bogue Chitto) to Pearl River, La. In this river reach, the coefficient was determined at each gaging site and at the mouth of each major tributary (fig. 3).

Flood-frequency discharges for 11 sites on the Pearl River are shown in table 1 and are plotted with drainage area in figure 4. The discharges for seven of these sites were agreed upon in 1980 by the U.S. Geological Survey and the U.S. Army Corps of Engineers, Mobile District, following the April 1979 flood. These sites were re-analyzed to include record through the 1988 water year, but analyses indicated that no revisions were warranted. For the other four sites (Burnside, Lena, Meeks Bridge, and Rockport), the records were extended using correlations with the nearest long-term station in accordance with Appendix 7 of Bulletin 17B (IACWD, 1982). The flood-frequency discharges for each site were divided by the appropriate basin-shape coefficient (fig. 3) to determine the discharges for an average-shaped basin (fig. 5).

If an estimate of a discharge for a specific frequency is needed for an ungaged site on the Pearl River, it is necessary to: (1) determine the drainage area, (2) obtain the discharge for an average-shaped basin from figure 5, and (3) multiply by the appropriate basin shape coefficient from figure 3.

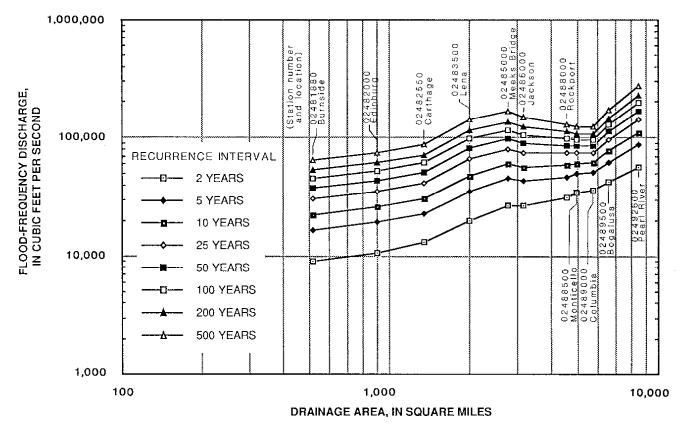


Figure 4.--Relation of flood-frequency discharge to drainage area for the Pearl River main stem.

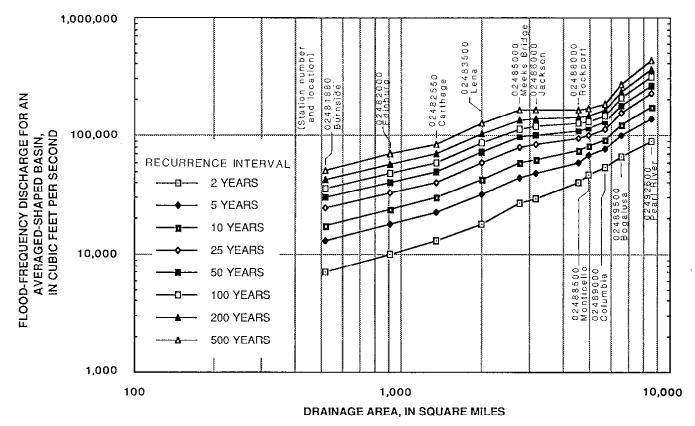


Figure 5.--Relation of flood-frequency discharge for an average-shaped basin to drainage area for the Pearl River main stem.

REGIONAL FLOOD-FREQUENCY ESTIMATES FOR RURAL STREAMS

Streamflow gaging records are available at only a small percentage of the stream sites where flood-frequency estimates are needed. Regionalization procedures are necessary to transfer flood-characteristic information from gaged to ungaged sites. Regional flood-frequency estimates also improve accuracy at a gaged site by weighting with station estimates, assuming that the station estimate is independent of the regional estimate. Regionalization procedures generally define relations between flood-frequency characteristics and explanatory drainage basin variables for gaged streams that are representative of similar streams in a specific class or region. Regionalization procedures have been the subject of much research through the years, and methods used by the U.S. Geological Survey have evolved as a result.

Graphical index-flood regionalization procedures were used by Wilson and Trotter (1961) for estimating flood magnitudes with recurrence intervals from 1.2 to 50 years for separate regions of the State. This detailed analysis included an adjustment factor for basin shape. Index-flood procedures were also presented by Patterson (1964) for streams in the lower Mississippi River basin and by Barnes and Golden (1966) for streams in the South Atlantic Slope and eastern Gulf of Mexico basins, Ogeechee River to the Pearl River, including parts of Mississippi. Continuing research by the U.S. Geological Survey led to the use of ordinary-least-squares (OLS) regression procedures to estimate T-year floods directly from drainage basin or climatic explanatory variables (Thomas and Benson, 1970). Regional T-year flood estimators for recurrence intervals from 2 to 100 years were determined using OLS procedures and were reported by Colson and Hudson (1976) for streams statewide, and by Landers (1985) for streams in the Lower Mississippi River Alluvial Plain. Recent developments in the regionalization of flood characteristics have centered on accounting for the deficiencies in the assumptions of OLS regression when applied to hydrologic variables.

Regional estimators of annual peak flood magnitude were computed in this study for recurrence intervals from 2 to 500 years. The maximum recurrence interval was 50 years in the report by Wilson and Trotter (1961) and 100 years in the report by Colson and Hudson (1976). Maximum recurrence interval is increased in this report because of changes in design standards requiring estimates of the 200-year and 500-year recurrence

intervals and not because of an improvement in the confidence given to the accuracy or methods.

Generalized-Least-Squares Regression

Two significant assumptions of OLS that usually are violated when estimating T-year floods are: 1) the errors are statistically uniform (homoscedastic), and 2) the observations are statistically independent in the sample. The error of T-year flood estimates varies from stream to stream with the length of record used to make the estimates. Also, T-year flood estimates may be correlated between streams experiencing similar climatic conditions and having similar drainage basin characteristics.

A procedure for estimating regional flood frequencies recently has been proposed by Stedinger and Tasker (1985 and 1986) that uses a weighting matrix to account for the time-sampling error and the cross-correlation of flood characteristics between sites. This procedure is called generalized-least-squares regression (GLS).

Cross-correlation between observations is estimated as a function of distance between gaged sites. The correlation-distance function is estimated from gages having long, concurrent record periods and in this study, is estimated from station pairs having concurrent record periods in excess of 30 or 50 years, depending on the region within the State. GLS regression also requires matrices of the mean, standard deviation, and skew associated with the matrix of log-transformed station T-year flood estimates. The standard deviation and skew matrices should be independent of the residual errors of the regional estimators in order that the model error can be quantified from the total residual error. In this study, regional estimates of the mean and standard deviation were computed using OLS regression of station mean and standard deviation against log-transformed drainage area and slope. The independent matrix of skews was estimated by using the matrix of estimates of population skew coefficients at each station.

Because GLS procedures compute and account for the time-sampling error and cross correlation of the observed T-year values, the total error of prediction of the regression equation may be divided into time-sampling errors arising from data limitations and model error arising from model limitations. Stedinger and Tasker (1985) used Monte Carlo simulations to

prove that, "In situations where the available streamflow records at gaged sites are of different and widely varying length and concurrent flows at different sites are cross-correlated, the GLS procedure provided more accurate parameter estimates, better estimates of the accuracy with which the regression model's parameters were being estimated, and almost unbiased estimates of the variance of the underlying regression model's residual errors," as compared with OLS or weighted-least-squares procedures.

Explanatory Variables

Regional flood-frequency estimators provide a means of extending the information gained at gaged locations to ungaged locations. Station T-year flood estimates were regressed on a range of potential explanatory variables including: drainage area, channel slope, channel length, mean basin elevation, basin shape factors, mean annual precipitation, and precipitation intensity. This testing was also performed on subgroups of the whole-sample group of sites, according to drainage area, region, and recurrence interval. Significant explanatory variables are drainage area, channel slope, and channel length. Logarithms of discharge have an approximately linear relation with logarithms of the selected basin characteristics, as shown in figure 6. The inverse relation of channel length, as a basin shape factor to discharge, is also illustrated in figure 6.

Regional Boundaries

An underlying assumption of regional flood-frequency relations is that the relation between T-year flood discharges and basin characteristics is similar for each stream and may be generalized for all streams represented in the data sample. This assumption was tested by comparing the regression residuals (observed value minus predicted value) between the whole-sample groups and selected subgroups. Tested subgroups were selected on the basis of major drainage basin boundaries, known regional variations in flood characteristics, and drainage area. Hypothesis tests were used to compute the probable equivalence between the whole-sample group and subgroups of the mean and median OLS regression residuals of the 10-year flood estimate.

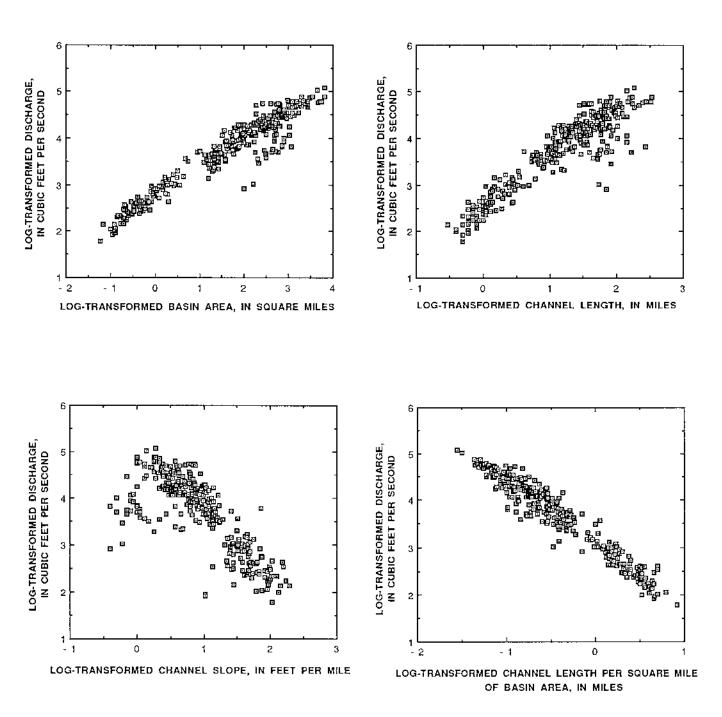


Figure 6.--Relation of the 10-year discharge to basin characteristics.

Figure 7 shows the characteristics of the OLS residuals for sites within the major drainage basin subgroups for an equation computed from the whole-sample group of sites outside the Delta region. These comparisons were also made using unweighted GLS residuals with similar results.

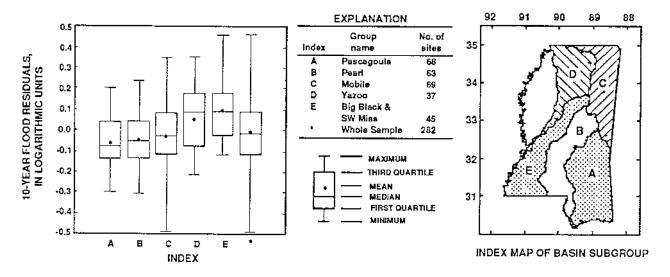


Figure 7. - - Characteristics of the 10-year flood residuals for the drainage basin subgroups and the whole-sample group of sites outside the Delta region.

Flood characteristics were determined to be non-homogeneous among four subgroups. Three of these subgroups are defined by geographic boundaries and one by drainage area magnitude. Regional flood-frequency equations were computed for each subgroup. Urbanized drainage basins were also analyzed separately.

Selection of the Appropriate Flood-Frequency Equation

Techniques for estimating the magnitude of floods with recurrence intervals from 2 to 500 years in Mississippi are provided in this report. If flood-frequency information is needed at a gaged site, it should be obtained from table 1. If the gage is not listed in table 1, the user must decide whether the appropriate estimate is obtained by weighting the station and regional estimates or from the unweighted station estimate (as when a stream is regulated or otherwise regionally unrepresentative.)

If flood-frequency information is needed at an ungaged site or if a regional estimate is needed to weight with a station estimate, then the appropriate regional flood-frequency equation must be selected. A user would select: (1) the Delta equations, if the stream is in the Delta; (2) the GT800 equations, if the stream is outside the Delta with drainage area greater than 800 square miles (GT800); or (3) the East or West equations, based on stream-site location (fig. 1). In figure 1, regions 1, 2, and 3 are the East, West, and Delta regions, respectively. The Delta and West boundary is crossed by stream basins sloping westward down the abrupt, dissected escarpment. For ungaged sites located in the Delta part of these basins, it is suggested that two discharges be estimated for each frequency by assuming all of the basin lies in each region and then averaging the discharges by areal weight. Drainage basins affected by urbanization should be estimated using the equations presented from the report by Sauer and others (1983), with the appropriate rural estimating equation.

The accuracy for each flood-frequency equation may be measured by using the standard error of prediction. The standard error of prediction for each equation is shown in table 2.

Table 2.--Standard error of prediction for each flood-frequency regression equation [GT800, basins in the eastern or western regions with areas greater than 800 square miles]

Recurrence interval,	Standa	ard error of pregion, in		or each
in years	East	West	Delta	GT 800
2	34	35	34	22
5	27	31	34	19
10	26	30	36	17
25	27	31	38	16
50	29	32	38	15
100	31	34	40	15
200	34	36	42	16
500	38	39	45	17

Delta

The most significant flood-characteristic boundary in Mississippi is between the Delta and the remainder of the State. Wilson and Trotter (1961) and Landers (1985) presented this region of the State as a separate hydrologic area with a unique flood-frequency relation. Landers (1985) presented regional flood-frequency equations for recurrence intervals from 2 to 100 years, based on data from 30 gaging stations in this region in Mississippi, Louisiana, and Arkansas. Of these 30 sites, only 6 are located in Mississippi (table 1). Comparisons of the 10-year flood regression residuals of the subgroup of 30 Delta streams to the whole-sample (statewide) group of 312 streams confirm the uniqueness of the Delta region. The statistical characteristics of the residuals are shown in figure 8.

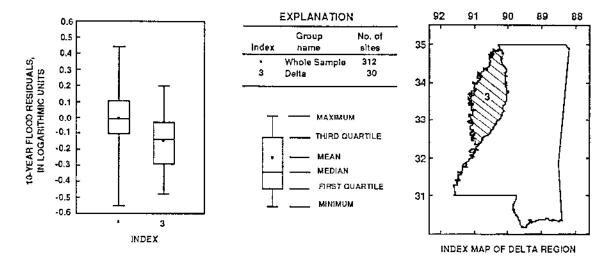


Figure 8. — Characteristics of the 10-year flood residuals for the whole-sample (statewide) group and for the Delta region.

The null hypothesis that the mean of the residuals is equal to the whole-sample mean (zero) was rejected at a 1-percent significance level, using the Student's t-test. Significant additional data have not been collected since 1985, because 19 of these 30 sites were discontinued. Therefore, the equations from Landers (1985) are repeated here without alteration. Those equations were computed using OLS regression procedures, and were checked in this

analysis. The recurrence interval was extended to 500 years using OLS regression procedures. The equations are as follows:

$$Q_2 = 171 (A)^{0.87} (S)^{0.25} (L)^{-0.52}$$
 (7)

$$Q_5 = 192 (A)^{0.93} (S)^{0.37} (L)^{-0.54}$$
 (8)

$$Q_{10} = 205 (A)^{0.96} (S)^{0.42} (L)^{-0.56}$$
(9)

$$Q_{25} = 224 (A)^{0.99} (S)^{0.48} (L)^{-0.58}$$
 (10)

$$Q_{50} = 232 \,(A)^{1.00} \,(S)^{0.52} \,(L)^{-0.57} \tag{11}$$

$$Q_{100} = 236 \text{ (A)}^{1.00} \text{ (S)}^{0.57} \text{ (L)}^{-0.55}$$
 (12)

$$Q_{200} = 243 \text{ (A)}^{1.00} \text{ (S)}^{0.60} \text{ (L)}^{-0.54}$$
 (13)

$$Q_{500} = 249 \text{ (A)} 1.00 \text{ (S)} 0.64 \text{ (L)} -0.52$$
 (14)

where

- Q_T is the estimated peak discharge, in cubic feet per second, for a recurrence interval of T years;
- A is the contributing drainage area, in square miles;
- s is the channel slope, in feet per mile, defined as the difference in altitude between points located at 10 and 85 percent of the main channel length divided by the channel length between the two points, as determined from topographic maps; and
- L is the main-channel length, in miles, from the point of discharge to the drainage divide as measured in 0.1 mile increments on topographic maps. At a stream junction, the branch draining the largest area is considered the main channel.

GT800

Streams outside the Delta were analyzed for flood characteristic homogeneity over the range of drainage areas (fig. 9). Comparisons of subgroups of OLS residuals indicate that flood estimates are over-predicted on stream basins larger than about 800 mi² and smaller than about 1 mi² (fig. 10). The subgroup of basins larger than 800 mi² was the most statistically different. When this subgroup of 33 sites was removed, the new whole sample (249 sites) was representative of the small drainage area sites (fig. 11). Equations for sites with drainage areas greater than 800 mi² (GT800) were computed using GLS procedures and are as follows:

$$Q_2 = 131 (A)^{0.97} (S)^{0.21} (L)^{-0.47}$$
 (15)

$$Q_5 = 382 (A)^{0.90} (S)^{0.22} (L)^{-0.48}$$
 (16)

$$Q_{10} = 668 \text{ (A)}^{0.87} \text{ (S)}^{0.21} \text{ (L)}^{-0.49}$$
(17)

$$Q_{25} = 1260 (A)^{0.84} (S)^{0.18} (L)^{-0.52}$$
(18)

$$Q_{50} = 1950 (A)^{0.83} (S)^{0.15} (L)^{-0.55}$$
(19)

$$Q_{100} = 2890 \text{ (A)}^{0.83} \text{ (S)}^{0.12} \text{ (L)}^{-0.59}$$
 (20)

$$Q_{200} = 4050 \text{ (A)}^{0.82} \text{ (S)}^{0.09} \text{ (L)}^{-0.63}$$
(21)

$$Q_{500} = 6070 \text{ (A)}^{0.83} \text{ (S)}^{0.06} \text{ (L)}^{-0.68}$$
(22)

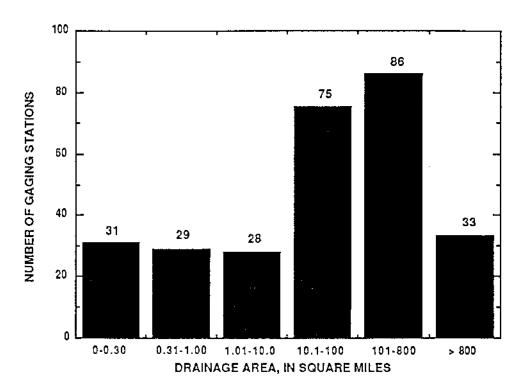
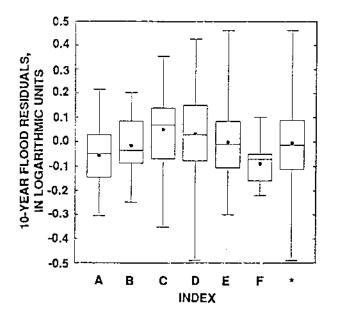
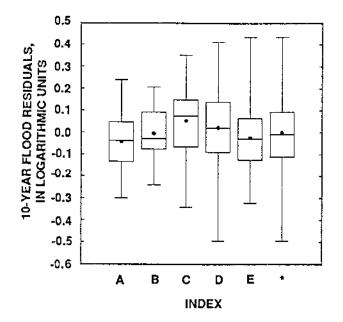


Figure 9.- Distribution of drainage area for 282 gaging stations outside the Delta region.



	EXPLANATION	
	Drainage area,	No. of
Index	In square miles	sites
Α	0 - 0.30	31
В	0.31 - 1.00	29
С	1.01 - 10.0	28
D	10.1 - 100	75
E	101 - 800	86
F	> 800	33
*	Whole Sample	282
	MAXIMUM THIRD QUAR MEAN MEDIAN FIRST QUAR MINIMUM	

Figure 10.--Characteristics of the 10-year flood residuals for drainage area subgroups and the whole-sample group of sites outside the Delta region.



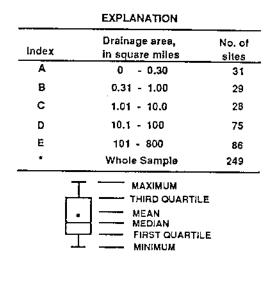


Figure 11.--Characteristics of the 10-year flood residuals for drainage area subgroups and the whole-sample group of sites outside the Delta region having drainage areas less than or equal to 800 square miles.

East

The whole sample of 249 sites for stream basins outside the Delta region and less than or equal to 800 mi² was also analyzed for flood-characteristic homogeneity. Residuals of the 10-year flood estimate were compared by major basin subgroup. The 10-year flood tended to be over-predicted in the Pearl, Pascagoula, and Mobile River basins (fig. 7). These basins were combined and are referred to as the East region (fig. 12). The small areas of the Hatchie and Tennessee River basins located in Mississippi were included in the East region. The null hypothesis that the mean of the residuals from the 174 sites in the East region is equal to the whole sample mean (zero) was

rejected at a 1-percent significance level using the Student's t-test. Equations for the East region were computed using GLS procedures and are as follows:

$$Q_2 = 296 (A)^{0.81} (S)^{0.03} (L)^{-0.36}$$
(23)

$$Q_5 = 406 (A)^{0.84} (S)^{0.07} (L)^{-0.35}$$
(24)

$$Q_{10} = 482 (A)^{0.85} (S)^{0.09} (L)^{-0.34}$$
 (25)

$$Q_{25} = 577 (A)^{0.85} (S)^{0.10} (L)^{-0.32}$$
(26)

$$Q_{50} = 648 \text{ (A)}^{0.85} \text{ (S)}^{0.11} \text{ (L)}^{-0.31}$$
(27)

$$Q_{100} = 716 \text{ (A)}^{0.85} \text{ (S)}^{0.11} \text{ (L)}^{-0.30}$$
(28)

$$Q_{200} = 786 \text{ (A)} \, {}^{0.85} \, (\text{S})^{0.12} \, (\text{L})^{-0.29}$$
 (29)

$$Q_{500} = 874 \text{ (A)}^{0.85} \text{ (S)}^{0.12} \text{ (L)}^{-0.28}$$
(30)

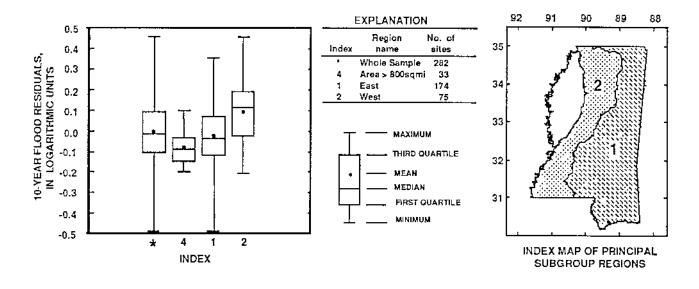


Figure 12.-- Characteristics of the 10-year flood residuals for the whole-sample group of sites outside the Delta region and for the principal subgroup regions.

West

The 10-year flood tended to be under-predicted for streams in the Yazoo (upstream of the Delta), Big Black, and southwest Mississippi drainage basins when the whole-sample of 249 sites for stream basins outside the Delta region and less than or equal to 800 mi² was analyzed for flood-characteristic homogeneity (fig. 7). These basins were combined and are referred to as the West region of the State (fig. 12). The null hypothesis that the mean of the residuals from the 75 sites in the West region is equal to the whole-sample mean was rejected at a 1-percent significance level using the Student's t-test. Equations for streams in the West region were computed using GLS procedures and are as follows:

$$Q_2 = 66.2 (A)^{0.88} (S)^{0.51} (L)^{-0.11}$$
 (31)

$$Q_5 = 94.7 \text{ (A)}^{0.93} \text{ (S)}^{0.51} \text{ (L)}^{-0.15}$$
(32)

$$Q_{10} = 122 (A)^{0.96} (S)^{0.49} (L)^{-0.19}$$
(33)

$$Q_{25} = 164 \text{ (A)}^{0.99} \text{ (S)}^{0.47} \text{ (L)}^{-0.24}$$
(34)

$$Q_{50} = 197 \text{ (A)}_{1.00} \text{ (S)}_{0.45} \text{ (L)}_{-0.26}$$
(35)

$$Q_{100} = 230 \text{ (A)}_{1.00} \text{ (S)}_{0.44} \text{ (L)}_{-0.25}$$
(36)

$$Q_{200} = 262 \text{ (A)} 1.00 \text{ (S)} 0.42 \text{ (L)} -0.25$$
(37)

$$Q_{500} = 305 (A)^{1.00} (S)^{0.41} (L)^{-0.25}$$
(38)

REGIONAL FLOOD-FREQUENCY ESTIMATES FOR URBANIZED STREAMS

Data have been collected in Mississippi on eight urban streams for which the period of actual flood data has been one of relatively constant urbanization. A preliminary analysis of the flood data on four of these streams in the Jackson area was reported by Wilson (1966). Due to the limited data, equations were not developed for this report, but a comparison was made between station frequency discharges (table 1) and discharges computed from the seven-parameter equations developed by Sauer and others in 1983. Those equations were developed using all available U.S. Geological Survey urban drainage basin data throughout the United States. Seven of the Mississippi urban sites were included in this nationwide analysis, which used flood data through the 1977 water year. The seven-parameter equations and definitions, excerpted from Sauer and others (1983), are as follows:

			Average standard error of prediction, in percent	
UQ ₂ =	=	$2.35 \mathrm{A}^{0.41} \mathrm{SL}^{0.17} \mathrm{(RI2+3)}^{2.04} \mathrm{(ST+8)}^{-0.65} \mathrm{(13-BDF)}^{-0.32} \mathrm{IA}^{0.15} \mathrm{RQ}_2^{0.47}$	±38	(39)
UQ5 =	=	$2.70 {\mathsf A}^{0.35} {\mathsf {SL}}^{0.16} (RI2+3)^{1.86} (ST+8)^{-0.59} (13\text{-BDF})^{-0.31} {\mathsf {IA}}^{0.11} {\mathsf {RQ5}}^{0.54}$	±37	(40)
UQ ₁₀ =	=	2.99A ^{0.32} SL ^{0.15} (RI ₂₊₃)1.75(ST+8)-0.57 (13-BDF)-0.30 _{IA} 0.09 _{RQ10} 0.56	8 ±38	(41)
UQ ₂₅ =	-	$2.78 \text{A}^{0.31} \text{SL}^{0.15} (\text{RI}_{2+3})^{1.76} (\text{ST}_{+8})^{-0.55} (13-\text{BDF})^{-0.29} \text{IA}^{0.07} \text{RQ}_{25}^{0.66}$) ±40	(42)
UQ ₅₀ =	=	$2.67A^{0.29}SL^{0.15}(RI2+3)^{1.74}(ST+8)^{-0.53}(13-BDF)^{-0.28}IA^{0.06}RQ_{50}^{0.62}$	2 ±42	(43)
UQ ₁₀₀ =	=	$2.50 \text{A}^{0.29} \text{SL}^{0.15} \text{(RI2+3)}^{1.76} \text{(ST+8)}^{-0.52} \text{(13-BDF)}^{-0.28} \text{IA}^{0.06} \text{RQ}_{100}^{0.6}$	3 <u>+44</u>	(44)
UQ ₅₀₀ =	=	$2.27 \text{A}^{0.29} \text{SL}^{0.16} (\text{RI2+3})^{1.86} (\text{ST+8})^{-0.54} (13\text{-BDF})^{-0.27} \text{IA}^{0.05} \text{RQ}_{500}^{0.6}$	3 ±49	(45)

where

UQT is the urban peak discharge, in cubic feet per second, for the recurrence interval of T years;

- A is the contributing drainage area, in square miles;
- SL is the main channel slope, in feet per mile, measured between points which are 10 percent and 85 percent of the main channel length upstream from the study site (for sites where SL is greater than 70, 70 is used in the equations);

- RI2 is rainfall intensity, in inches, for the 2-hour 2-year occurrence (U.S. Weather Bureau, 1961).
- ST is basin storage, the percentage of the drainage basin occupied by lakes, reservoirs, swamps, and wetlands (inchannel storage of a temporary nature, resulting from detention ponds or roadway embankments, is not included in the computation of ST);

BDF is the basin development factor;

- IA is the percentage of the drainage basin occupied by impervious surfaces, such as houses, buildings, streets, and parking lots; and
- RQT is the peak discharge, in cubic feet per second, for an equivalent rural drainage basin in the same hydrologic area as the urban basin, and for recurrence interval of T years.

The basin development factor (BDF) describes the conditions of the drainage system. The following description of the BDF and how it is computed is a quotation from Sauer and others (1983).

The most significant index of urbanization that results from this study is a basin development factor (BDF), which provides a measure of the efficiency of the drainage system. This parameter, which proved to be highly significant in the regression equations, can be easily determined from drainage maps and field inspections of the drainage basin. The basin is first divided into thirds. Then, within each third, four aspects of the drainage system are evaluated and each assigned a code as follows:

1. Channel improvements.--If channel improvements such as straightening, enlarging, deepening, and clearing

are prevalent for the main drainage channels and principal tributaries (those that drain directly into the main channel), then a code of 1 is assigned. Any or all of these improvements would qualify for a code of 1. To be considered prevalent, at least 50 percent of the main drainage channels and principal tributaries must be improved to some degree over natural conditions. If channel improvements are not prevalent, then a code of zero is assigned.

- 2. Channel linings.--If more than 50 percent of the length of the main drainage channels and principal tributaries has been lined with an impervious material, such as concrete, then a code of 1 is assigned to this aspect. If less than 50 percent of these channels is lined, then a code of zero is assigned. The presence of channel linings would obviously indicate the presence of channel improvements as well. Therefore, this is an added factor and indicates a more highly developed drainage system.
- 3. Storm drains, or storm sewers.--Storm drains are defined as enclosed drainage structures (usually pipes), frequently used on the second tributaries where the drainage is received directly from streets or parking lots. Many of these drains empty into open channels; however, in some basins they empty into channels enclosed as box or pipe culverts. When more than 50 percent of the secondary tributaries within a subarea (third) consists of storm drains, then a code of 1 is assigned to this aspect; if less than 50 percent of the secondary tributaries consists of storm drains, then a code of zero is assigned. It should be noted that if 50 percent or more of the main drainage channels and principal tributaries are enclosed, then the aspects of channel improvements and channel linings would also be assigned a code of 1.

4. Curb-and gutter streets.--If more than 50 percent of a subarea (third) is urbanized (covered by residential, commercial, and/or industrial development), and if more than 50 percent of the streets and highways in the subarea are constructed with curbs and gutters, then a code of 1 would be assigned to this aspect. Otherwise, it would receive a code of zero. Drainage from curb-and-gutter streets frequently empties into storm drains.

The above guidelines for determining the various drainage-system codes are not intended to be precise measurements. A certain amount of subjectivity will necessarily be involved. Field checking should be performed to obtain the best estimate. The basin development factor (BDF) is the sum of the assigned codes; therefore, with three subareas (thirds) per basin, and four drainage aspects to which codes are assigned in each subarea, the maximum value for a fully developed drainage system would be 12. Conversely, if the drainage system were totally undeveloped, then a BDF of zero would result. Such a condition does not necessarily mean that the basin is unaffected by urbanization. In fact, a basin could be partially urbanized, have some impervious area, have some improvement of secondary tributaries, and still have an assigned BDF of zero.

The BDF is fairly easy index to estimate for an existing urban basin. The 50-percent guideline will usually not be difficult to evaluate because many urban areas tend to use the same design criteria, and therefore have similar drainage aspects, throughout. Also, the BDF is convenient for projecting future development. Obviously, full development of the drainage system and maximum urban effects on peaks would occur when BDF = 12. Projections of full development or intermediate stages of development can usually be obtained from city engineers.

The nationwide equations were used to estimate the 2-year, 10-year, and 100-year floods for the eight Mississippi urban sites. Four additional basin characteristics needed in these equations are presented in table 3. The nationwide equation estimates are compared with the observed station estimates in figure 13. The comparison is also made on the basis of the root-mean-square error (RMS) of the estimating equation, computed as:

$$RMS = \sqrt{(\overline{X})^2 + S^2}$$
 (46)

where \overline{X} is the average error and S is the standard deviation of the error. The root-mean-square error is considered an approximation of the standard error of prediction. Table 4 presents errors of prediction for the group of all eight sites and for the group of five sites in Jackson. For the Jackson sites, the minimum and maximum errors are significantly negative for the 2-year and both positive and negative for the 100-year recurrence interval. Average errors for the 10-year and 100-year recurrence intervals are similar to those reported for adjoining states by Sauer (1986).

Table 3.--Additional basin characteristics for urban streams in Mississippi

[BDF, basin development factor; RI2, rainfall intensity for the 2-hour 2-year occurrence; ST, basin storage; IA, impervious area]

Station number	Station name	BDF	RI2 (inches)	ST (percent)	IA (percent)
2473047	Gordon Creek at Hattiesburg	5	2.7	0	21
2485800	Eubanks Creek at Jackson	8	2.5	0	33
2485950	Town Creek at Jackson	7	2.5	0	29
2486100	Lynch Creek at Jackson	4	2.5	0	27
2486115	Three Mile Creek at Jackson	6	2.5	0	29
2486350	Caney Creek at Jackson	6	2.5	2	14
7289610	Bachelor Creek at Canton	2	2.4	0	10
7290910	Spanish Bayou at Natchez	4	2.6	0	27

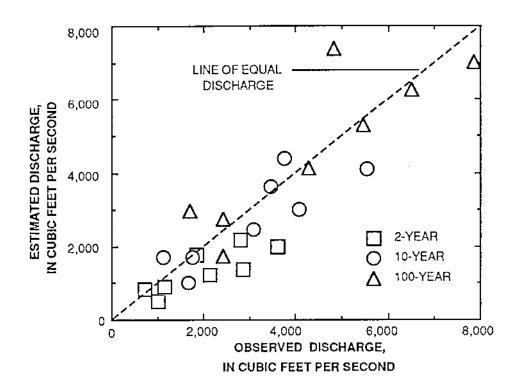


Figure 13.-- Relation of observed 2-year, 10-year, and 100-year urban peak discharge to peak discharge estimated from equation 39, 41, and 44.

Table 4.--Errors of prediction using the seven-parameter nationwide equations for urbanized streams in Mississippi

			Errors o	of prediction, i	n percent	
Sites	Recurrence interval, in years	Minimum	Maximum	Average	Standard deviation	Root-mean square error
All	2	-52	+14	-28	<u>+</u> 24	±37
(8 sites)	10	-39	+50	-6	±29	±29
	100	-29	+73	+11	±34	±36
Jackson	2	-52	-23	-42	±12	±44
(5 sites)	10	-39	+16	-19	±21	±28
	100	-29	+54	+2	±31	±31

With the limited data, the Student's t-test, at the 1-percent level of significance, indicates that the negative error for the 2-year recurrence interval is statistically significant when considering only the Jackson sites. However, for all sites combined, no bias in using the seven-parameter equations is proven. The RMS error for the 2-, 10-, and 100-year discharges for all eight sites (table 4) is somewhat lower than ±38, ±38, and ±44 percent, respectively, as reported in the nationwide study (Sauer and others, 1983); however, when considering only the Jackson sites, the RMS is higher for the 2-year discharge.

The seven-parameter nationwide equations can be used to estimate flood frequencies for an ungaged urbanized stream in Mississippi. However, the limited data, especially in Jackson, indicate that the 2-year to 10-year discharges may be significantly underestimated using the nationwide equations. This emphasizes the need for more peak runoff data for urbanized areas in Jackson and throughout Mississippi.

LIMITATIONS OF REGIONAL FLOOD-FREQUENCY ESTIMATES

Limitations always exist for an estimate obtained from a regional flood-frequency equation. The most significant known limitations are listed in the following sections. To avoid introducing large errors in estimates, the user should become aware of possible basin projects which may alter flood flows.

Rural Streams

The following limitations should be observed when using the regional equations in this report for estimating flood-frequency discharges on a rural Mississippi stream because the equations:

- are not considered to be representative for basins outside the range of characteristics (explanatory variables) in the sample set for each region (table 5);
- should not to be used for sites where a significant part of the basin is affected by regulation and (or) channelization;

- do not apply to estuarine sites near the mouths of coastal streams at which unusual flood discharges result from hurricane tides flowing into or out of storage;
- should be used with caution near the mouths of streams
 draining into larger streams because the larger stream may cause
 critical stages and discharges at the recurrence interval in
 question; or
- may not be fully representative of the steep loess "bluff" hills, bordering parts of the Delta, and the flat coastal region of the State, extending roughly 20 mi inland from the Gulf of Mexico, due to the limited data in these areas.

Table 5.--Characteristics of explanatory variables used in regression calculations for basins in the East and West regions with areas less than or equal to 800 square miles, basins in the Delta, and basins in the East or West regions with areas greater than 800 square miles (GT800)

[Area, in square miles; Channel slope, in feet per mile; Channel length, in miles]

Region	Basin characteristic	Mean	Median	Minimum	Maximum
East				•	
	Area Channel slope Channel length	146 25.4 23.0	40.3 10.2 12.2	$0.10 \\ 1.5 \\ 0.4$	799 170 123
West					
	Area Channel slope Channel length	131 28.8 18.2	35.3 10.9 12.3	0.06 2.3 0.3	654 192 70.7
Delta					
	Area Channel slope Channel length	389 2.1 65.7	300 1.0 56.0	$0.11 \\ 0.4 \\ 0.5$	1,170 10.6 269
GT800					
	Area Channel slope Channel length	2,368 2.1 134	1,650 1.8 110	831 0.7 49.1	6,590 4.4 338

Urbanized Streams

The seven-parameter nationwide equations for estimating floodfrequency discharges on an urbanized Mississippi stream apply when the basin and climatic variables are within the following ranges:

- A -- 0.2 to 100 mi²
- SL -- 3.0 to 70 ft/mi
- RI2 -- 0.2 to 2.8 in
- ST -- 0 to 11 percent
- BDF -- 0 to 12
- IA -- 3 to 50 percent

The maximum value for SL for use in the equations is 70 ft/mi; although numerous drainage basins used in the development of the equations had SL values up to 500 ft/mi. If values for the variables are outside these ranges, the standard error may be considerably higher than for sites where all variables are within the specified range (Sauer and others, 1983).

SUMMARY

This report provides techniques for estimating the magnitude of floods with recurrence intervals from 2 to 500 years for streams in Mississippi. Estimates of flood magnitude are presented for 330 streamflow-gaging stations. Flood-frequency discharges for seven of the eleven streamflow-gaging stations on the Pearl River, which were agreed upon in 1980 by the U.S. Geological Survey and the U.S. Army Corps of Engineers, Mobile District, are included. A graphical relation of flood-frequency discharge to drainage area for the Pearl River main stem, with an adjustment for basin shape, is also presented.

Regression analyses were used to define relations between floodfrequency characteristics and explanatory drainage basin variables for 282 rural streamflow-gaging stations, which are representative of similar streams in a specific class or region. To improve accuracy of the regression equations, the State was divided into four subgroups, three defined by geographic boundaries and one by drainage area magnitude. Generalized-least-squares regression, which defines more accurate estimates of regression coefficients and model error than ordinary-least-squares regression, was used in the analyses of three subgroups. The Delta subgroup was analyzed using ordinary-least-squares regression, and because relatively little data have been collected since 1985, previously published equations are presented with extension to 500 years. The regression analyses indicated that size of drainage area, slope of the main channel, and length of the main channel were the most significant basin characteristics that affect the magnitude and frequency of floods for all four subgroups. Regression equations presented for the four subgroups may be used to estimate the magnitude and frequency of floods for ungaged rural stream sites in the State. If the drainage area at an ungaged site is within 50 percent of the drainage area at a gaged site on the same stream, the flood-frequency estimate can be extrapolated using the flood frequency at the gaged site weighted with the regional estimate at the ungaged site using equation 6.

Only eight sites were available for which the period of record was one of relatively constant urbanization. For these sites, a comparison was made between frequency discharges computed from the record and discharges computed from the seven-parameter nationwide equations described previously. When considering only the five sites in Jackson, the 2-year discharge appears to be under-estimated using the nationwide equations; however, for all sites combined, no bias in using the nationwide equations is proven. Therefore, the seven-parameter nationwide equations are presented and can be used to estimate the magnitude and frequency of floods for an ungaged urban stream in the State.

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Table 1.—Flood quantiles, standard error of estimate, and other information for gaging stations [Notes are explained at end of table; sd, standard deviation; lACWD, Interagency Advisory Committee on Water Data; sqmi, square miles; ft/mi, feet per mile; mi, miles; region: (1) East, (2) West, (3) Delta, (4) G7800, beains in the East or West regions with areas greater than 800 sqmi]

	Station no.	V	Period of	top line Peak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)							
			record.	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
1	2429900 note: * region 1	Big Bxcwn Creek near Booneville, MS mean= 3.270; sd= 0.245; skew0.400 area= 27.1sqmi; slope= 15.7ft/mi; length= 10.1mi	1953-88	1,940 10	3,070	3,850 10	4,870 12	5,680 14	6,460 17	7,330 20	8,370 23
2	note: *	Little Brown Creek near New Site, MS mean= 3.318; sd= 0.366; skew= 0.027 area= 42.2sqmi; slope= 9.6ft/mi; length= 12.4mi		2,260 20	4,390 19	6,080 20	8,160 23	9,750 25	11,200 28	12,930 30	14,900 34
3	ente: *	TOTILE SECTION OF THE	1967-87	212 14	399 14	567 16	812 19	1,020 22	1,200 24	1,420 27	1,670 31
4	note: *b	Mackeys Creek near Dennis, MS 	1939-79	 							
5	note: *	Red Bud Creek near Moores Mill, MS mean= 2.878; sd- 0.203; skew= 0.005 area= 15.7sqmi; slope= 22.7ft/mi; length= 6.1mi	1975-88	840 13	1,380 13	1,850 14	2,590 17	3,230 20	3,870	4,590 25	5,450 29
6	note: *al	Tombigbee Rivor near Mariotta, MS bgj mean- 4.015; sd- 0.210; skew= 0.261 area- 308 sqmi; slope- 6.1ft/mi; length= 24.1mi	1938-51 1968-77 1955	12,000	20,000	27,300 	41,000	52,500 	64,500 		100,000
7	ante: c	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1976-88	61.4 8	821 8	975 9	1,210 11	1,420 14	1,640 16	1,880 18	2,170 22
8	rote: c	Cummings Creek near Fulton, MS mean= 2.795; sd= 0.226; skew= 0.359 area= 19.1sqml; slope= 8.5ft/mi; length= 7.0mi	1975-88	761 15	1,430 16	2,110 18	3,17G 21	4,030 24	4,840 27	5,740 30	6,780 34
9	note: *al	Tombigbee River near Fulton, MS ogj mean= 4.260; sd= 0.249; skew0.021 arca= 612 sqmi; slope= 3.5ft/mi; length= 42.2mi		19,900	30,300	38,900	52,500 	64,000	78,000		122,000
3,0	notie: *al	Tombigbee River at Boans Ferry near Fulton, MS dgj mean= 4.236; sd= 0.189; skew= 0.050 area= 706 sqmi; slope= 2.9ft/mi; length= 49.7mi	1938-47 1927	21,200	3],900 	41,300 	56,000 	68,40C 	83,500		130,000
11	note: *	Bull Mountain Creek at Tremont, MS mean= 3.710; sd= 0.301; skew= -0.003 area= 136 sqmi; slope= 8.2ft/mi; length- 33.6mi	1941-64 1973-83	5,060 14	9,000 14	12,100 16	16,200 18	19,300 21	22,400 24	26,000 27	30,300 31
12	2432900 note: * region 1	Red Boot Creek near Fulton, MS mean= 1.812; sd= 0.194; skew- 0.149 area= 0.13sqmi; slope= 89.3ft/mi; length= 0.8mi	1955-75	65 10	96 11	121 13	154 16	181 18	206 21	236 24	269 28

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

— ·· Map		Station Station name and description			top line Poak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)							
no.	no.			of record				error of 25-year	•		•	
13	note: *	Bull Mountain Creek hear Smithville, MS mean= 4.016; sd= 0.325; skew= -0.073 area= 336 sqmi; slope= 1.9ft/mi; longth=		1941-88 1927	10,100 11	18,100	24,100	31,700 15	37,300 18	42,900 20	49,100 23	57,100 27
14	note: *ab	Tombigbee River at Bigbee, MS ogj mean- 4.473; sd= 0.234; skew= 0.080 area= 1,230 sgmi; slope= 1.8ft/mi; length=	79.0mi	1937-58 1964-88 1927,62	30,00C 	43,000	58,600 	80,000	99,000	121,000		183,000
15	note: *	fown Creek at Tupelo, MS mean= 3.910; sd= 0.210; skew= 0.061 area- lll sqmi; slope= 8.2ft/mi; length=	20.2mi	1939-46 1949-88		12,000	14,900 9	18,500 12	21,300 14	24,200 16	27,400 19	31,500 22
16	note: *	Tishomingo Creek near Saltillo, MS mean= 3.474; sd= 0.166; skew= 0.084 area= 30.1sqmi; slope= 11.8ft/mi; length=	16.Cmi	1950-63	2,810 11	3,910 11	4,700 13	5,730 16	6,570 19	7,390 21	8,380 24	9,550 28
17	nate: *	Euclautubba Creek at Saltillo, MS mean= 3.447; sd= 0.162; skew= 0.091 area= 19.1sqmi; slope= 9.7ft/mi; length=	9.2mi	1949-75	2,690 8	3,680 9	4,350 10	5,180 13	5,840 15	6,480 18	7,223	8,140 24
18	note: *	Truck stop Ditch near Tupelo, MS mean= 2.218; sd= 0.187; skew= -0.011 area= 0.22scm1; slope= 46.2ft/mi; length=	0.7mi	1955-72	159 10	224 11	268 12	319 15	359 18	395 20	438 23	486 27
19	note: *	Town Creek at Eason Boulevard at Tupelo, MS mean- 4.094; sq= 0.237; skew= -0.197 area- 233 sqmi; slope= 6.9ft/mi; length=		1971-88	11,800 13	18,500 13	23,230 14	29,000 16	33,500 19	37,800 22	42,700 24	48,800 28
20	note: *	Cow Pike Pass near Tupelo, MS mear= 2.103; sd= 0.147; skew= 0.298 area= 0.14sqmi; slope= 52.9ft/mi; longth=	0.6mi	1955-83	122 8	163 9	190 10	225 13	253 16	278 19	308 21	344 25
21	note: *	Clear Branch near Tupelo, MS mean= 2.201; sd- 0.203; skew- 0.145 area= 0.75sqmi; slope= 47.5tt/mi; length=		1955-83	161 9	249 10	320 12	422 15	508 17	590 20	686 23	799 26
22	note: *	Town Creek near Verona, MS mean= 4.152; sd= 0.244; skew= 0.275 area= 271 sqm; slope= 6.2tt/mi; length=	27.6mi	1941-61	13,000 12	20,700 13	26,400 15	33,600 19	39,200 21	44,600 24	51,100 27	59,000 31
23	note: *	Coonewah Creek at Shannon, MS mean= 3.732; sd= 0.244; skew= 0.193 area= 53.1sqmi; slope= 9.7tt/mi; length=	20.8mi	1952-85 1939	4,930 11	7,650 12	9,520 13	11,800 17	13,500 19	15,100 22	17,100 25	19,500 29
24	note: *	Cotton Gin Branch near Tupelo,MS mean= 2.075; sd- 0.221; skew= -0.092 area= 0.30sqmi; slope= 40.7ft/mi; length=		1955-76	120 11	183 11	229 13	287 15	334 18	375 20	425 23	481 27

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map no.	Station Station name and description no.		Period of record	botto	m line	Standard	error of	T-year f.	lood estí:	feet per : mate (per: 200-year	cent)
					0 1000	10 7001		,		1	
25	2435930 Shell Crock near Tupelo, MS note: * mean= 1.900; sd= 0.203; skow= 0.478 rogion l area= 0.20sqm1; slope= 28.3ft/m1; length=	0.8mi		78 9	120 10	154 12	203 16	243 19	279 22	322 25	371 29
26	2436000 Chiwapa Creek at Shannon, MS note: *abg mean= 4.188; sd= 0.192; skcw= C.027 region 1 area= 145 sqmi; slope= 7.4ft/mi; length=	24.0mi	1950-88				28,600 12		37,100 17	41,600 20	47,700 24
	2436500 Town Creek near Nettloton, MS note: * mean= 4.451; sd= 0.197; skew= 0.615 region 1 area= 620 sgm1; slope= 6.9ft/m1; longth=	38.2mi	1940-88 1927	26,200 7		49,300 10	63,100 I4	74,200 17		98,500 23	116,000 27
28	2437000 Tombigbee River near Amory, MS note: *abgj mean= 4.568; sd- 0.238; skew= 0.281 region 4 area= 1,930 sgml; slope= 1.8ft/mi; longth=	83.0mi	1938-88 1927 1892		62,900 	83,000	113,000	140,000	170,000		260,000
	2437300 Mattubby Creek noar Aberdeen, MS note: * mean= 3.845; sd= 0.206; skew= -0.500 region 1 arca= 92.2sgm; slope= 6.6ft/mi; length=	20.1mi	1952-88 1937				14,400 10		17,500 15	19,100 17	21,100
30	2437500 Tombigbee River at Abordoon, MS note: *abgj mean= 4.465; sd= 0.264; skew= 0.180 region 4 area= 2,170 sgmi; slope= 1.8(t/mi; longth= 1	01.0mi	i 909-82 1983 1892	33,000	53,700	70,000 	96,900 	120,000	145,000		220,000
31	2437550 Nichols Crock tributary noar Quincy, MS note: * mean= 2.178; sd= 0.208; skew= 0.090 region l area= 0.54sqmi; slope= 90.4ft/mi; length=	1.3mi	1966-88	154 11	238 11	305 13	399 16	478 19	550 21	638 24	736 28
	2437600 James Croek at Abordeen, MS note: * mean= 3.524; sd- 0.205; skow= -0.062 region 1 area= 28.4sqmi; slope= 71.8ft/mi; length=	9.4mi	1963-88 1961 1948	3,250 10	4,850 10	6,010 11	7,510	8,730 16	9,880 19	11,300 21	12,900 25
33	2438300 Buttahatchec River below Hamilton, AL note: * mean= 4.192; sd= 0.203; skew= -0.270 region 1 area= 277 sqm1; slope= 6.2ft/mi; length=	44.8mi	1951-88	15,200 8		26,700 9	32,000 11	35,900 13	39,800 15	43,900 18	49,200 21
	2439000 Buttahatchee River near Sulligent, AL note: * mean= 4.135; sd= 0.266; skew= -0.453 region 1 area= 472 sqmi; slope= 4.8ft/mi; length=				22,630 8	28,400 8	35,600 11	40,800 13	46,000 15	51,600 18	58,500 21
	2439400 Buttahatchee River near Abordoon, MS noto: * mean= 4.279; sd= 0.266; skew= -0.439 region 1 area- 799 sqmi; slope= 4.1ft/mi; length-	83.6mi	1967-88	18,800 13	30,800 12	39,000 13	49,300 15	57,030 18	64,700 21	73,500 24	84,300 28
36	2439500 Buttahatchec River near Calcdonia, MS note: * mean- 4.189; sd= 0.283; skew= -0.178 region 4 area= 83% sqmi; slope- 3.9ft/mi; length=	93.1mi	1929-32 1938-51	15,100 13	25,900 13		44,100 13			66,200 15	80,700 16

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description	Period of record	botto	m line	Standard	ear flood error of	T-year f	lood esti	mate (per	cent)
				2-year	5-year	10-year	25-year	50-year	100-year	200-year	300-year
37	2439800 note: * reglor 1	Cowboll Creek near Houlka, MS mean- 2.222; sd- 0.203; skew0.088 alea- 0.46sqmi; slope- 25.7ft/mi; length= 0.9m:	1955-76	169 10	252 10	311 12	388 14	449 17	506 19	570 22	616 26
38	note: *	Chuquatonchee Creek near Okalona, MS mean= 3.604; sd= 0.252; skew= 0.425 area= 68.5sqmi; slope= 8.8ft/mi; length= 13.4mi	1964-88 1951,53	3,850 13	6,50C 14	8,710 16	11,700 20	14,000 22	16,200 25	18,800 28	21,800 32
39	2439997 note: * region 1	Chuquatonchee Creek tributary near Trebloc, MS mean= 2.504; sd= 0.114; skew= -0.118 area= 0.74sgmi; slope= 50.0ft/mi; length= 1.4m;	1966-84	317 6	397 7		512 9	562 11	609 13	662 15	726 18
40	2440000 note: *	Chuquatonchee Creek near Egypt, MS mean= 3.971; sd= 0.269; skew= -0.104 area= 167 sqmi; slope= 6.1ft/mi; length= 27.2m:	1950-88	9,040 10	14,900 11	19,000 12	24,300 14		32,000 19	36,300 22	
47.	2449400 note: * region l	<pre>Boulka Creek near McCondy, MS mean= 3.990; sd= 0.250; skew= 0.130 area= 189 sqmi; slope= 5.1ft/mi; length= 25.8mi</pre>	1963-88	9,240 11	14,900 12	19,100 14	24,500 17	28,600 20	32,700 22	37,400 25	43,300 29
42	moto * *	Chuquatonchee Creek near West Point, MS mean= 4.225; sd= 0.258; skew= -0.090 area= 505 sqm; slope= 1.8ft/mi; length= 44.8m;				34,100 10	43,700 13	51,000 15	58,300 18		76,500 24
43	2440600 note: * region 1	Line Crock near Mabon, MS mean= 3.216; sd= 0.267; skew= 0.159 area= 4.76sgmi; slope= 32.2ft/mi; length= 5.3m:	1952-88	1,470 11	2,310 11	2,850 13	3,420 16	3,850 19	4,220 22	4,710 24	5,270 28
44	2440800 note: * region 1	<pre>Trim Cane Creek near Starkville, MS nean- 3.696; sd- 0.186; skew= -0.462 area= 44.9sqmi; slope= 13.8ft/mi; length= 13.8m.</pre>	1952-88 1940	4,960 8	6,960 7	8,170 8	9,55 0 9	10,500 11	11,500 14	12,500 16	13,700 19
45	note: *	Tibbee Crock near Tibboo, MS mean= 4.669; sd= 0.276; skew0.611 area= 926 sqm; slope= 3.8ft/m; length= 54.8m;	1940-88 1929-30 1927	29,100			71,300 9	81,600 1:	94,100 12	101,000	723,000 15
46		Sand Creek tributary near Mayhow, MS mean= 2.242; sd= 0.233; skew= -0.357 area= 0.44sqmi; slope= 13.4ft/mi; length= 1.1mi		177 11	269 11	329 12	400 14	453 17	500 19	553 22	614 26
47	2441300 note: * region 1	Catalpa Creek at Mayhew, MS mean= 3.860; sd= 0.267; skew= -0.077 area= 98.0scmi; slope= 9.1ft/mi; length= 17.2mi	1963-87	6,850 13	11,200 13	14,200 14	17,900 17	20,730 19	23,400 22	26,600 25	30,400 29
48	2441500 note: *ak region 4	Tombigboo River at Columbus, MS ogj mean= 4.699; sd= 0.243; skew= 0.214 area= 4,460 sgmi; slope= 1.4tt/mi; length= 143.0mi	1892-99 1900-88	58,100 	94,200	124,000	168,000	207,000	279,000		400,000

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	5tation	Station name and description		Period	top line Peak T-year flood magnitude (cubic feet bottom line Standard error of T-year flood estimate						feet per :	second)
no.	no.			of record			10-year					
	note: *	Luxapallila Creck at Steens, MS mcan= 3.845; sd= 0.185; skcw= -0.019 area= 309 sqmi; slope= 5.8ft/mi; length=	56.9mi	1940-88	7,030 6	10,200	12,600 8		18,500 12		24,300 16	
50	2443500 note: * region 1	<pre>Luxapallila Creck near Columbus, MS mean= 4.218; sd= 0.243; skew= -0.585 area= 715 sqmi; slope= 9.0ft/mi; length-</pre>	65.2m1	1973-88 1968-69 1961-65	17,180 11	26,800 9	33,200 10		47,700 15	54,190 18	61,300 20	70,100 24
51.	note: *	Mayo Slough tributary near Columbus, MS mean= 2.211; sd= 0.164; skew= -0.138 area= 0.24sqmi; slope= 46.0ft/mi; length=		1965-75	158 12	215 12	253 13	300 15	337 18	370 21	410 23	4 54 27
	note: *	Cedar Creek near Brooksville, MS mean= 2.513; sd= 0.107; skew= -0.055 area= 0.49sqmi; slope= 21.9ft/mi; length=		1965-84	321 6	395 6	440 7	493 9	533 11	572 13	614 15	665 18
53	2444000 note: * region l	Coal Fire Creek near Pickensville, AL mean- 3.410; sd- 0.342; skew= 0.181 area= 126 sqmi; slope= 5.5ft/mi; length-	36.4mi	1955-80	2,800 15	5,710 15	8,340 17		15,000 23	17,900 26	21,100 29	25,100 33
54	2447220 note: * region 1	Bogue Fallah tributary near Ackerman, MS mean= 2.085; sd= 0.288; skew= -0.326 area= 0.34sqmi; slope= 67.5ft/mi; length=	1.1mi	1966-84	128 15	214 14	276 15	351 17	410 20	460 23	522 26	588 30
55	rote: *	<pre>Lawson Branch near Bethoden, MS mean= 2.467; sd= 0.328; skew= -0.219 area= 1.09sqmi; slope= 32.0ft/mi; length=</pre>		1965-77	292 19	501 18	649 18	818 21	945 23	1,050 26	1,190 29	1,330 33
56	2447340 note: * region 1	Cypress Creek tributary at Bradley, MS mean= 2.127; sd= 0.30e; skew= -0.280 area= 0.60sqm1; slope= 27.9ft/m1; length=	1.9mi	1966-77	148 18	253 17	330 18	426 20	501 23	564 26	644 29	727 33
	2447500	Noxubee River near Brooksville, MS mean= 3.968; sd= 0.361; skew0.047 area= 446 sqmi; slope= 3.4ft/mi; length=		1940-73 1979		18,300 14	25,500 15	35,000 18	42,100 21	49,300 23		67,400 30
58	note: *	Hashuqua Creek near Macon, MS mean- 3.511; sd- 0.358; skew= 0.218 area= 96.2sqmi; slope= 11.9ft/mi; length-	28.0mi	1951-70 1976,79	3,320 17	6,640 17	9,440 19			18,500 27	21,700 30	25,300 34
	region l	Noxubee River near Macon, MS mean= 4.123; sd= 0.334; skew= 0.035 area= 768 sqmi; slope= 2.5ft/mi; length=	90.6mi	1892,1927	13,200 11		34,700 23				81,400 23	
60	2448508 note: * region 4	Noxubee River near Geiger, AL mean= 4.102; sd= 0.271; skew= 0.446 area= 1.100 sqmi; slope= 0.7ft/mi; length=	140.0mi	1929-88	11,900 8	20,200 9	27,000 11	35,800 12	43,90C 13		58,300 14	

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map no.	Station no.	Station name and description		Period of record			Peak T-yo Standard					
					2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
61	note: *	Flat Scooba Creek tributary near Scooba, MS mean- 2.117; sd- 0.194; skew- 0.387 area- 0.44sqmi; slope= 44.01t/mi; length=	Į.Omj	1967-88	130 10	201 11	260 13	345 17	416 20	481 22	559 25	646 30
62	note: *	Hamiiton Branch near Dekalb, MS mean= 2.543; sd= 0.171; skow= 0.018 area= 0.97sqmi; slopc= 45.8ft/mi; length=	1.7mi	1965-77	339 11	475 12	573 13	698 16	800 19	892 21	1,010 24	1,130 28
63	note: *	Little Okatubba Croek near Quitman, MS mean= 2.913; sd= 0.207; skew0.003 area= 4.35sqmi; slope- 41.2ft/mi; length=	3.5mi	1966-84	804 11	1,210 12	1,520 13	1,910 16	2,230 18	2,530 21	2,890 24	3,300 28
64	cote: *	Loaf River noar Raleigh, MS mean= 3.667; sd= 0.319; skew= -0.123 aroa= 143 sqmi; slope= 3.3ft/mi; length=	39.8mi	1940-43 1957-88 1856,1900	4,700 12	8,490 12	11,400 13	15,200 16	18,200 19	21,100 21	24,300 24	28,300 28
65	note: *	Leaf River near Taylorsville, MS mean= 3.989; sd= 0.167; skew= 0.706 area= 459 sqmi; slope= 3.3ft/mi; length=	57.4mi	1968-88 1961 1856,1900	9,380 8	13,800	18,000 12	24,700 16	30,600 20	36,700 23	43,600 26	52,400 30
66	ngte: *	Oakohey Creek at Mize, MS mean= 3.728; sd= 0.265; skew= 0.055 area= 185 sqmi; slope- 4.3ft/m1; length=	36.lmi	1942-49 1968-88 1961	5,380 12	9,180 12	12,230 14	16,600 17	20,000 19	23,500 22	27,400 25	32,300 29
67	note: *	Leaf River near Collins, MS mean= 4.160; sd= 0.247; skew= 0.307 area= 743 sqmi; slope= 3.0ft/mi; length=	68.7mi	1939-88 1900 1856	14,000 8	23 , 400 9	31,200 11	42,700 14	52,100 17	62,300 20	73,000 23	87,700 26
68	not.e: *	Biq Creek tributary near Laurel, MS mean= 2.117; sd= 0.163; skow= -0.003 area= 0.17sqmi; slope= 82.0ft/mi; length=	0.6mi	1966-84	128 9	175 9	207 11	247 13	279 16	308 18	343 21	382 24
69	note: *	Boule Creck near Sanford, MS mean= 3.878; sd= 0.329; skew= 0.518 area= 262 sqmi; slope= 7.1ft/mi; length=	40.4mi	1968-88 1961	7,190 16	13,800 17	19,500	26,600 22	32,000 25	37,000 28	43,100 31	50,300 35
70	note: *	Boule Creek near Eattlesburg, MS mean= 3.795; sd= 0.313; skew= 0.651 area= 304 sqmi; slope= 6.5ft/mi; length=	51.0mi	1939-88 1930	5,960 11	11,700 12	17,200 15	25,500 19	32,100 22	38,500 25	45,900 28	55,000 32
71	note: *	Okatoma Creek tributary at Mt. Clive, MS mean= 2.038; sd- 0.293; skew= 0.403 area= 0.33sqm1; slope= 63.0ft/m1; length-		1965-77	112 17	200 18	270 20	353 23	419 25	470 28	540 31	608 35
72	note: *	Okatoma Creek tributary no.2 near Collins, MS moan= 2.136; sd= 0.245; skew- 0.391 area- 0.21sqm; slope= 82.0ft/mi; longth=		1967-84	128 13	201 14	255 17	317 20	367 23	406 26	461 29	515 33

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description	Period of record			Peak T-yo Standard					
			record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
	note: *	Leaf River at Hatticsburg, MS mean= 4.418; sd= 0.240; skew- 0.444 area 1,750 sqmf; slope= 2.5ft/mi; length= 111.0mi	1905-88 1900	25,000 6		54,000 9		86,500 11	103,000	111,000	137,000 15
74	2473047 note: dg region l	Gordon Creek at Hattiesburg, MS mean= 3.282; sd= 0.195; skew= 0.556 area= 8.83sqm1; slope- 21.9ft/m1; length= 7.3mi	1969-88	1,840 9	2,740 11	3,470 14	4,540 20	5,460 26	6,500 32	7,660 39	9,430 49
	note: *	Tallahala Creek at Waldrup, MS mean= 3.702; sd= 0.271; skew= 0.129 area= 102 sqm; slope= 4.1ft/m; length= 24.6m1	1969-88 1961	4,820 13	8,070 14	10,500 16	13,700 19	16,100 22	18,500 24	21,200 27	24,500 31
	note: *	Tallahattah Creek near Waldrup, MS mean= 3.217; sd= 0.343; skew= 0.098 arca= 30.4sqm; slope=11.0ft/m; length= 13.0m1		1,710 16	3,280 16	4,560 17	6,220 20	7,500 23	8,670 26	10,100 29	11,700 33
	note: *	Tallahala Creek at Laurel, MS mean= 3.751; sd= 0.343; skew= -0.078 area= 238 sqm1; slope= 3.2ft/m1; length= 56.6mi	1938-88 1900,20	5,740 11	10,900	15,100 13	20,800 35	25,300 18	29,800 20	34,700 23	41,100 27
-	note: *d	Tallahala Creek tributary no.2 at Laurei, MS mean= 2.684; sd= 0.170; skew= 0.115 area= 1.52sqmi; slope= 24.8ft/mi; length= 3.0mi	1974-84	480 13	670 14	802 17	974 23	1,110 28	1,240 35	1,380 41	1,580 51
	noto: *	Tallahoma Creek tributary at Lake Como, MS mean= 3.050; sd= 0.186; skew= -0.001 area= 3.21sqmi; slope= 31.5ft/mi; length= 3.4mi	1964-88	1,060 9	1,490 10	1,780 11	2,100 14	2,360 16	2,580 18	2,870 21	3,200 25
	nole: *	Tallahala Creek near Runnelstown, MS nean- 3.915; sd~ 0.259; skew- 0.355 area= 612 sqmi; slope= 2.5ft/mi; length= 102.0mi	1940-88 1980,20		14,100 10	19,400 12	27,600 15	34,500 18	41,900 21	50,100 24	61,100 28
81	2474600 note: * region 1	Bogue Homo near Richton, MS near= 3.891; sd- 0.240; skew= 0.307 area= 344 sqmi; slope= 3.7ft/mi; longth- 64.9mi	1971-88 1941-43	7,590 12	12,700	17,000 16	23,100 19	28,100 22	33,100 24	38,80C 27	45,900 32
	note: *	Buck Creek near Runnelstown, MS nean= 3.371; sd= 0.195; skew= 0.046 area= 20.8sqmi; slope= 13.5ft/mi; length= 11.8mi	1951-88	2,290 8	3,340 8	4,080 10	5,040 12	5,800 15	6,530 17	7,380 19	8,430 23
	note: *	Leaf River at Beaumont, MS nean= 4.556; sd= 0.225; skew= 0.480 area+ 3,010 sqmi; slope- 2.2ft/mi; length= 153.0mi	1941-61 1972-76 1900	34,600 11	54,8C0 12	71,300 13	90,700 13	308,000 14	128,000 14	134,000 15	164,000 17
84	2475000 note: * region 4	<pre>Leaf River near McLain, MS nean= 4.577; sd= 0.226; skew= 0.345 area= 3,500 sgml; slope= 1.9ft/ml; length= 169.0ml</pre>	1940-88 1900,38	36,700 8		74,800 10			135,000 13		176, 0 00 16

Table].—Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description		Period of		top line Peak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)							
				record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year	
	note: *	Waterfall Branch near McLain, MS mean= 2.457; sd- 0.215; skew= 0.253 ursa: 0.60sqmi; slope-73.32t/mi; length-	1.0mi	1955-88	277 9	424 10	534 12	679 15	796 17	901 20	1,030 23	1,180 27	
86	note: *	Little Rock Creek tributary near Little Rock, mean= 1.707; sd= 0.402; skew= 0.305 area= 0.22sqmi; slope= 170 ft/mi; length=		1965-84	64.0 19	141 19	21G 2G	294 23	360 25	409 28	478 31	541 35	
-	note: *	Tarlow Creek near Newton, MS mean= 3.262; sd= 0.135; skew- 0.099 area= 16.1sqmi; slope- 12.5ft/mi; length=	7.0mi	1953~70 1979	1,800 8	2,390 8	2,800 10	3,380 12	3,850 14	4,330 17	4,850 19	5,520 23	
	note: *	Chunky River near Chunky, MS mean= 3.934; sd= 0.321; skew= 0.158 area= 369 sgmi; slope- 5.3ft/mi; length=	39.8mi	1939-88	8,570 11	16,300 12	22,900 13	32,300 16	40,000 19	47,500 22	56,200 25	67,200 29	
	note: *b:	Okatibbee Crock near Meridian, MS i mean= 3.680; sd= 0.380; skew= 0.163 area= 235 sqmi; slope= 3.5ft/mi; length=	46.1ml	1938-72								==	
	note: *	Sowashee Creek at Meridian, MS mean= 3.437; sd= 0.294; skew= 0.128 area= 52.lsqmi; slope= 12.0ft/mi; length=		1936-45 1949-88	2,750 10	4,960 11	6,820 13	9,420 16	11,500 18	13,600 21	16,000 24	19,000 28	
	note: ag	Okatibbee Creek at Arundel, MS mean= 3.749; sd= 0.269; skew= 0.300 area= 342 sqmi; slope= 3.5ft/mi; length=	51.3mi	1969-88 1961	5,430 15	9,330 18	12,600	17,600 30	22,000 38	27,100 47	32,900 57	41,800 72	
	note: *	Chickasawhay River at Enterprise, MS mean= 4.179; sq- 0.285; skew- 0.011 area= 918 sqmi; slope= 4.4ft/mi; length-	58.0mi	1905-88 1900 1871		27,500 8	37,700 8	52,800 10	66,600 11	82,000 12	91,500 13	115,000	
	note: *	Souchlovie Creek hear Baxter, MS mean= 2.706; sd= 0.170; skew= 0.231 area= 1.14sqmi; slope= 46.5ft/mi; length=	1.7mí	1964-88	485 8	674 9	804 11	968 14	1,100 26	1,220 19	1,370 22	1,540 25	
	note: *	Powers Creek near Rose Bill, MS mean= 2.436; sd= 0.168; skew- 0.307 area= 0.45sqmi; slope- 107 ft/mi; length=	l.lml	1964-84	260 9	360 10	431 12	521 15	596 18	663 21	748 23	842 27	
	note: *	Souchlovie Creek near Pachuta, MS mean= 3.713; sd= 0.437; skew= -0.015 area= 174 sgmi; slope= 4.6ft/mi; length=	35.lmi	1956-70 1938,49 1900	5,34C 20	10,900 19	15,100 20	20,200 23	23,900 25	27,400 28	31,500 31	36,500 35	
	note: *	Pachuta Creek at Pachuta, MS mean= 3.266; sd= 0.407; skcw= -0.039 area= 23.2sqmi; slope= 14.5ft/mi; length=	11.9mi	1952-70 1938,49	1,800 19	3,45C 19	4,660 20	6,050 22	7,110 25	8,030 27	9,200 30	10,500 34	

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description	Period of	to botto	p line m line	Peak T-ye Standard	ear flood error of	magnitud T-year f	e (cubic locd esti	feet per mate (per	second) cent)
			record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
	note: *	Shubuta Creck near Shubuta, MS mean= 3.526; sd= 0.344; skew= 0.033 area= 75.5sqmi; slope- 7.1ft/mi; length= 28.lmi	1963-88	3,290 16	6,150 16	8,360 17	11,200 20	13,200 22	15,200 25	17,600 28	20,400 32
	note: *	Chickasawhay River at Shubuta, MS mean= 4.286; sd= 0.799; skew= 0.257 area= 1,460 sqmi; slope= 2.4ft/mi; length= 99.6mi	1905-64 1972-88 1900	19,200 9	34,600 10	47,600 11	64,200 12	78,400 13	93,800 13	101,000	126,000 16
	note: *	Chickasawhay River near Waynesboro, MS mean= 4.217; sd= 0.245; skew= 0.374 area= 1,650 sqmi; slope= 2.1ft/mi; length- 120.0mi	1937-88 1930	16,600 9	28,300 9	39,000 11	54,700 12	68,800 13	84,100 13	91,100 14	114,000
100	note: *	Buckatunna Creek near Donham, MS mean= 3.795; sd= 0.209; skew= 0.179 area= 492 sgmi; slope= 3.0ft/mi; length= 82.8mi	1972-88	6,500 12	10,700 13	14,500 14	20,400 18	25,400 20	30,600 23	36,500 26	44,000 30
	note: *	Buckatunna Creek at Denham, MS mean= 3.907; sd= 0.204; skew= 0.641 area= 505 sqm1; slope= 2.9ft/mi; length= 87.8mi	1938-49 1900,20 1951,61,79	7,890 12	12,900 13	17,700 16	25,300 20	31,500 23	37,800 26	44,900 29	53,700 33
102	2478500 note: * region 4	Chickasawhay River at Leakesville, MS mean= 4.374; sd= 0.216; skew= 0.519 area= 2,690 sgmi; slope- 1.6ft/mi; length= 184.0mi	1938-88 1900,16	23,100	36,500 8	48,400 10	65,300 11	80,500 12	96,800 13	104,000	128,000 16
103	note: *	Granny Branch at Piave, MS mean= 2.369; sd= 0.193; skew0.165 area= 0.69sqmi; slope- 47.8ft/mi; length= 1.2mi	1967-84	236 31	346 11	423 12	521 14	602 17	676 19	762 22	860 26
104	note: *	Pascagoula River at Merrill, MS mean= 4.806; sd- 0.218; skew 0.249 area- 6,590 sqmi; slope= 1.9ft/mi; length= 184.0mi	1905-88 1900	62,800 6	97,030 6	123,000 7	157,000 9	186,000 10	218,000	233,000	283,000 14
105	note: *	Big Creck hear Lucedalo, MS mean- 2.978; sd- 0.429; skew= 0.117 area= 21.2sqmi; slope= 22.0ft/mi; length= 6.0mi	1952+70	1,290 20	2,780 19	4,130 20	5,840 23	7,140 25	8,270 28	9,660 31	11,200 35
106	note: *	Blown Pine Creek near Hatticsburg, MS mean= 2.532; sd= 0.362; skew= 0.353 area= 1.92sqm1; slope=51.9ft/m1; length= 3.3mi	1966-77 1955,83	337 20	628 20	843 21	1,390 24	1,280 26	1,430 29	1,650 31	1,860 36
107	note: *	Black Creek near Purvis, MS mean= 3.511; sd= 0.315; skew= 0.497 area= 171 sqmi; slope- 6.2ft/mi; length= 35.6m1	1957-70 1974	3,650 18	7,550 19	11,200 20	16,200 23	20,000 26	23,600 28	27,700 31	32,500 35
108	note: *	Black Crock near Brooklyn, MS mean= 3.957; sd= 0.230; skew= 0.815 area= 355 sqmi; slope= 4.7ft/mi; length= 56.3mi	1971-88 1961	8,440 13	14,200	19,400 17	26,800 21	32,700 24	38,300 27	44,900 30	52,700 34

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description	Period Of			Peak T yo Standard					
			record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
109	2479138 note: * region l	Walls Creek tributary near Brooklyn, MS mean= 2.197; sd- 0.298; skew= 0.429 area= 0.37scm1; slope= 61.2ft/ml; length- 1.3ml	1966-77 1983	146 18	245 18	316 20	395 23	459 25	508 28	580 31	648 35
110	note: *	Walls Creek near Brooklyn, MS mean= 3.050; sd- 0.480; skew= 0.345 area= 22.6sqmi; slope= 12.0ft/mi; length- 11.0mi	1951-70	1,280 21	2,830 21	4,050 22	5,433 24	6,450 27	7,340 29	8,480 32	9,710 36
111	note: *	Cypress Creek near Janice, MS mcan= 3.537; sd= 0.273; skew= 0.693 area= 52.6sqmi; slope- 9.1ft/mi; length- 12.1mi	1967-88 1959	3,200 13	5,620 15	7,650 17	10,200 21	12,200 24	14,000 27	16,100 30	18,600 34
112	2479160 note: * region 1	Black Creek near Wiggins, MS moan= 4.146; sd= 0.226; skew= 0.792 area= 701 sqmi; slope= 3.0ft/mi; length= 103.0mi	1972-88 1959,61 1916	12,900 12	21,100	28,400 17	38,800 21	46,900 24	55,100 27	64,500 30	76,000 34
113	2479165 note: * region 1	Mosquito Branch at Benndale, MS mean= 1.859; sd= 0.291; skew= 0.288 area= 0.22sqmi; slope= 96.7ft/mi; length= 1.0mi	1955-77	74 14	135 15	185 17	250 20	303 23	347 25	403 28	461 32
114	note: *	Black Creek near Bonndale, MS moan= 4.081; sd- 0.291; skew= 0.198 arca= 753 sqm; slope= 2.5ft/mi; length- 123.0mi	1959-70 1949	11,700 17	21,000 17	28,600 19	38,700 22	46,600 24	54,400 27	63,400 30	74,700 34
115	2479180 note: * region 1	Red Crock at Lumberton, MS moan= 2.961; sd- 0.363; skew= 0.458 area= 15.7sqmi; slope= 13.9ft/mi; length- 7.4mi	1951-73	993 18	2,070 18	3,020 20	4,200 23	5,090 25	5,860 28	6,810 31	7,840 35
116	note: *	Rod Creek tributary near Wiggins, MS mean= 2.089; sd= 0.287; skew= 0.366 area= 0.22sqmi; slope= 48.0ft/mi; length= 1.0mi	1966-84	114 15	187 16	237 18	294 21	338 24	372 26	421 29	470 34
117	note: *	Red Crook near Wiggins, MS mean= 3.704; sd= 0.273; skow= 0.474 area= 177 sqmi; slope= 5.0ft/mi; length= 32.0mi	1952-70 1916,28 1948	4,980 13	9,000 15	12,600 17	17,600	21,500 23	25,200 26	29,600 29	34,800 33
118	note: *bi	Flint Creek near Wiggins, MS mean= 3.125; sd- 0.279; skew= 0.403 area= 24.9sqmi; slope= 13.4ft/mi; length= 8.3mi	1957-68 1948,53 1954								
119	note: *	Bluff Creek tribulary near Whites Crossing, MS mean= 2.616; sd= 0.275; skew= 0.311 area= 0.82sgmi; slope= 31.7ft/mi; length= 1.1mi	1966-77 1955	368 17	576 17	711 18	856 23	970 24	1,060 26	1,190 29	1,320
120	2479300 note: * region :	Red Crock at Vestry, MS mean= 3.962; sd= 0.233; skow= 0.614 area= 441 sqmi; slope= 2.9ft/mi; length= 76.lmj	1959-88	8,690 10	14,500 12	19,500 14	27,100 18	33,200 21	39,400 24	46,400 27	55,300 32

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description		Period of record						e (cubic : lood esti		
				recoru	2-year	5-year	10-year	25-year	50-year	100-year	200 ~ year	500-year
121	2479500 note: * region :	Escatawpa River near Wilmer, AL mear= 3.985; sd= 0.227; skew- 0.488 area= 511 sqm; slope= 2.7ft/mi; length=	55.Cmi	1946-75	9,440 10	15,700 11	21,200 14	29,830 17		44,100 23	52,200 26	
122	2479560 note: * region 1	Escatawpa River near Agricola, MS mear= 4.061; sd= 0.190; skew- 0.570 area= 562 sqm1; slope- 2.7ft/m1; length=	61.1mi	1974-86	11,100 12	17,600 14	23,400 16	32,200 20		46,600 26		
123	2479600 note: * region 1	Escatawpa River near Eurley, MS mean= 3.987; sd= 0.205; skcw= 0.291 area= 646 sqmi; slopc= 2.5ft/mi; length=	77.7mi	1958-70	9,850 13	16,100 14	21,700 16	30,300 19		44,400 25	52,400 28	
124	2480150 note: * region 1	Franklin Creek near Grand Bay, AL mear= 2.944; sd= 0.321; skcw= 0.094 area= 16.7sqmi; slopc= 13.5ft/mi; length-	7.8mi	1959-79	983 16	1,910 16	2,710 17	3,820 20	4,690 23	5,490 26	6,430 29	7,500 33
125	2480500 note: * region 1	Tuxachanic Creek near Biloxi, MS mear= 3.719; sd= 0.230; skew= 0.833 area= 92.4sqmi; slope- 6.8ft/mi; length-	26.lmi	1953-88 1966,48	4,773 9	7,660 11	10,100 14	13,400 18	15,900 21	18,300 24	21,200 28	24,700 32
	note: *	Biloxi River at Wortham, MS mean= 3.699; sd= 0.156; skew= 0.138 area= 96.2sqmi; slope= 7.3ft/mi; length=	29.6mi	1953-88 1948	4,910 6	6,740 7	8,050 8	9,846 11		12,8G0 15	14,400 17	16,600 20
127	2481130 note: * region l	Bilox: River near Lyman, MS mear= 4.068; sd= 0.193; skcw= 0.662 area= 251 sqmi; slope= 6.0ft/mi; length=	38.2mi	1964-88 1957	10,700 9		20,200 14	25,900 18			40,70C 27	47,400 31
128	2481400 note: * region 1	Wolf River near Poplarville, MS mear= 3.364; sd= 0.312; skew= 0.867 area= 71.0sqmi; slope= 8.1ft/mi; length=	20.6mi	1952-71	2,310 16	4,690 17	6,980 19	10,000 23	12,300 26	14,300 28		19,400 36
129	2481450 note: * region 1	Murdor Creek near Poplarville, MS mear 3.157; sd- 0.238; skew= 1.082 area 21.6sqmi; slope= 16.8ft/mi; length=	10.9mi	1952-70 1916,48 1836,76	1,340 12	2,340	3,310 17		5,780 25		7,860 31	9,110 35
130	2481500 note: * region 1	Wolf River at Lyman, MS mean= 3.902; sd= 0.252; skew- 0.674 area= 253 sqmi; slope- 5.4ft/ml; length=	47.6mi	1945-48 1965-70 1961	7,310 17	12,500 18	16,900 20	22,600 23	27,000 26	31,200 28	36,300 31	42, 200 36
1.31	2481505 note: * region 1	Mill Creek tributary near lizana, MS mean= 2.748; sd= 0.205; skew= 0.542 area= 2.29sqmi; slope-46.lft/mi; lengte-	2.2mi	1967-77	527 14	815 16	1,050 18	1,350 21	1,600 24	1,800 27	2,080 30	2,350 34
132	2481510 nole: *	Wolf River near Landon, MS mean= 3.993; sd= 0.163; skew= 0.283 area= 308 sqm; slope= 4.9ft/mi; length=		1971-88			16,500 12			28,500 21		38,500 27

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description		Period of			Poak T-yo Standard					
				record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
133	2481670 Bayou note: * mca region 1 area=	La Croix near Clerπont Harbor, MS n= 3.239; sd= 0.203; skew= 0.424 38.Csqmi; slope= 1.51t/ml; length=	↓8.9mi	1960-70	1,800 15	2,930 1.6	3,920 18	5,330 21	6,410 24	7,460 27	8,580 30	9,980 34
134	note: * mea	ga Creek near Noxapater, MS n= 3.478; sd= 0.348; skow= 0.148 58.6sqmi; slope= 6.7ft/mi; length=	19.0mi	1953-70 1974,79	2,950 17	5,520 17	7,490 19	9,900 22	11,700 24	13,400 27	15,500 30	17,800 34
135	note: * mea	ter Creek near Noxapater, MS n= 3.252; sd= 0.299; skew- 0.363 35.3sgmi; slope- 6.7ft/m1; length=	14.lmi	1952-70 1979	1,800 15	3,320 16	4,600 18	6,300 21	7,590 24	8,790 27	10,200 30	11,900 34
236	note: fa mea	River at Burnside, MS n= 3.951; sd= 0.314; skew= -0.100 520 sgmi; slope= 1.9ft/m1; length=	47.6mi	1981-88 1935,38 1939,62,79	9,040 9	16,500 9	22,400	30,900 14	37,900 17	45,500 20	53,800 24	65,600 29
337	note: * mea	uck Crock tributary near House, MS n= 1.963; sd= 0.251; skow= -0.173 0.20sqmi; slope= 97.8ft/mi; length-	0.6mi	1965-77 1979	97 15	157 15	201 16	257 18	303 21	341 24	390 27	439 30
138	note: *g mea	River at Edinburg, MS n= 4.023; sd= 0.309; skew0.100 904 sqmi; slope- 1.3ft/mi; length=	76.3mi	1909-88 1902 1878	10,700 9	19,300 9	26,100 10	35,800 14	43,800 17	52,500 20	61,800 24	75,200 29
139	note: * mea	Branch near Edinburg, MS n= 2.468; sd= 0.259; skew= -0.177 1.91sgmi; slope= 27.1ft/mi; length=	2.5mi	1965-84	313 13	52 <u>1</u> 13	680 11	892 17	1,070 19	1,220 22	1,410 25	1,620 29
1.40	note: * mea	ha Creek tributary at Wamba, MS n- 2.586; sd= 0.229; skew= -3.092 0.94sqmi; slope= 38.3ft/mi; length=	1.3m1	1964-84	359 12	549 12	680 13	839 16	96 4 18	0,070 21	1,210 24	1,350 28
141	note: * mea	ha Crook near Carthage, MS n= 3.767; sd= 0.345; skew0.209 309 sgmd; slope= 2.2ft/ml; length=	57.7mi	1938-70 1979	6,210 14	11,800 13	16,200 14	22,200 17	26,800 19	31,400 22	36,500 25	42,900 29
142	note: *fo mea	River near Carthage, MS n= 4.117; sc= 0.293; skew= -0.050 1,350 sgmi; slope= 1.4ft/mi; length-	97.5mi	1962-88 1932,38-39 1874,1900,0		23,100 15	30,900 17	42,100 22	51,300 27	61,300 33	72,000 39	87,500 48
143	note: * mea	ogue Creek tributary near Harpersville, n= 1.617; sd= 0.299; skew= -0.228 0.12sqmi; slope= 131 ft/mi; length-		1965-77	52 18	93 17	126 17	17¢ 20	206 23	236 25	273 28	310 32
	2483000 Tuscol note: * mea region l area=	amota Creek at Walnut Grove, MS n= 4.000; sd= 0.271; skew= -0.238 411 sqmi; slope= 4.1ft/m²; length=	35.4mi	1939-88 1900,02	10,400 9	17,300 9	22,600 10	29,700 12	35,300 15	41,000 17	47,200 19	55,100 23

Table 1.--flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map Station Station name and description no. no.		Period of record			Peak T-ye Standard					
		record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-yea
145 2483500 Pearl River near Lena, MS note: *fg mean= 4.302; sd= 0.296; skew= 0.000 region 4 area= 1,980 sgml; slope= 1.3ft/mi; length=	-]10.0mi	1937-53 1962,74 1902	20,000 16	35,600 17	48,100 20	66,200 27	81,400 33	98,000 40	116,000 47	143,000 59
146 2483890 Yockanookany River tributary near McCool, MS note: * mean= 2.147; sd= 0.333; skew= -0.179 region 1 area= 0.34sqmi; slope= 51.3ft/mi; length=		1965-88	142 15	252 15	330 16	42î 18	490 21	546 24	618 27	692 31
147 2484300 Yockanookany River near Kosciusko, MS note: * mean= 3.837; sd= 0.280; skew= 0.090 region l area= 303 sqmi; slope= 3.3ft/mi; length=	37.lmi	1938-88 1933	6,930 9	12,200 10	16,600 11	22,900 14	28,100 17	33,400 19	39,300 22	47,100 26
148	- 73.9mi	1938-88	7,540 9	12,900 9	17,200 11	23,600	28,800 16	34,300 18	40,20C 21	48,200 25
149	- 23.5m1	1971-87	3,570 12	5,400 12	6,760 13	8,620 16	10,100	11,600 21	13,300 24	15,300 28
150 2484750 Red Came Creek tributary near Pisgah, MS note: * mean= 1.779; sd= 0.201; skew= -0.064 region 1 area= 0.10sqmi; slope= 73.3ft/mi; length=		1965 88	61 10	91 10	113 11	142 14	166 16	187 19	212 22	240 25
151 2484760 Fannesgusha Creek noar Sand Hill, MS nole: * mean= 3.499; sd= 0.265; skew= 0.040 region 1 area= 52.3sqmi; slope= 7.8ft/mi; length=	- 12.2mi	1977-88	3,140 14	5,310 14	7,000 16	9,230 19	11,000 21	12,600 24	14,500 27	16,800 31
152 2485000 Pearl River at Mocks Bridge near Canton, MS note: *Ig mean= 4.435; sd= 0.267; skow= 0.050 region 4 area= 2,760 sqmi; slope= 1.1ft/mi; length=	- 144.3mi	1938-63 1979,83 1933	27,100 13	45,600 14	60,000 16	80,700 22	97,700 27	116,000 32	136,000 39	166,000 47
153 2485380 Hollybush Creek tributary no.1 near Pisgah, note: * mean= 2.357; sd= 0.155; skew= -0.100 region 1 area= 0.59sqm1; slope= 23.0ft/mi; length=			227 8	308 9	362 10	431 12	486 14	538 17	597 19	666 22
154 2485385 Hollybush Creek tributary no.2 near Pisgah, note: * mean= 2.183; sd= 0.175; skew= -0.078 region 1 area= 0.25sqmi; slope= 53.6ft/mi; length=		1965-77	149 11	209 12	250 13	302 16	343 18	380 21	425 24	174 27
155 2485392 Clear Creek tribulary near Pelahatchie, MS note: * mean= 1.921; sd= 0.165; skew= -0.092 region 1 area= 0.12sqmi; slope= 141 ft/mi; length-		1965-84	84 9	116 9	138 10	167 13	191 15	213 17	238 20	267 23
156 2485500 Pelahatchic Creek noar Fannin, MS note: * mean= 3.925; sd= 0.359; skew= -0.222 region 1 area= 206 sqmi; slope= 3.5ft/mi; length=		1938-39 1950-65	6,790 19	12,600 17	17,000 18	22,300 21	26,200 23	30,000 26	34,300 29	39,500 33

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map no.	Station no.	Station name and description		Period of			Peak T-ye Standard					
				record	2-year	5-year	10-year	25-year	50~year	100-year	200-year	500-year
	note: da	Purple Creek near Jackson, MS mean= 3.366; sd- 0.186; skew0.003 area= 6.12sqm1; slope=16.2ft/m1; lesgth=	6.5ml	1952~88	1,160 8	1,670 9	2,010	2,460 13	2,800 16	3,140 20	3,490 23	3,970 28
	note: *	<pre>Hanging Moss Creek tributary near Tougaloo, MS mean= 2.680; sd= 0.241; skew= 0.103 area= 3.56sqmi; slope= 20.9ft/mi; length=</pre>	4.9mi	1952-68	480 13	782 14	1,020 16	J,340 19	1,590 21	1,820 24	2,100 27	2,420 31
	note: *bd	Hanging Moss Creek at Jackson, MS ig mean= 3.386; sd= 0.162; skew= 0.022 area= 16.8sgmi; slope= 12.6ft/mi; length=	7.4mi	1953-88	2,430 7	3,330 8	3,930 9	4,690 12	5,260 14	5,830 17	6,410 20	7,190 25
	note: da	Eubarks Creek at Jackson, MS mean= 3.333; sd= 0.121; skew= 0.182 area= 5.19sqmi; slope= 23.9ft/mi; longth=	3.4mi	1953-88	2,140 5	2,710 6	3,090 7	3,560 9	3,910 12	4,260 14	4,620 17	5,090 20
	note: *	Neely Creek near Brandon, MS mean= 2.492; sd= 0.301; skow= 0.301 area= 1.09sqmi; slope= 40.1ft/mi; length=	1.4mi	1964-84	303 15	533 16	71C 17	920 21	1,080 23	1,220 26	1,390 29	1,570 33
162	note: do	Town Creek at Jackson, MS mean= 3.447; sd= 0.098; skew= 0.104 area= 11.4sqmi; slope= 14.2ft/m1; length=	6.7mi	1953-84 1914,21 1885	2,790 4	3,380 5	3,750 6	4,193 8	4,510 9	4,820 11	5,120 13	5,520 16
	note: ac	Poarl River at Cackson, MS mean= 4.430; sd= 0.252; skew= 0.050 area= 3,170 sqmi; slope= 1.0ft/mi; longtb= 1	77.0mi	1900-88 1874,81	26,80C 6	43,800 7	56,80C 8	75,000 11	90,000 13	106,000 16	123,000 19	148,000 23
	note: dq	lynch Creek at Jackson, MS maan= 3.558; sd- 0.145; skew= 0.000 area= 12.1sqmi; slope= 15.5ft/mi; length-	6.5mi	1953-88	3,620 6	4, 790 7	5,550 8	6,490 10	7,180 12	7,860 15	8,540 17	9,450 21
	note: da	Three Mile Creek at Jackson, MS mean= 2.999; sd= 0.181; skew= -0.271 area= 1.05sgmi; slope= 44.4ft/mi; length=	1.8mi	1962-78 1981-86	1,020	1,420	1,680 10	1,990 13	2,210 16	2,420 19	2,620 23	2,890 27
	note: *	Richland Creek tributary near Grandon, MS mean= 1.692; sd- 0.257; skew= -0.187 area= 0.12semi; slope= 60.5ft/mi; length=	0.6mi	1966-77	55 16	90 16	117 17	151 19	179 22	202 25	232 27	262 31
	note: dg	Cany Creek at Jackson, MS mean- 3.459; sd- 0.119; skew= 0.012 area= 8.38sqmi; slope= 19.8ft/mi; length=	5.4mi	1961-77	2,870 9	3,620 10	4,080 11	4,650 15	5,050 18	5,450 22	5,830 26	6,340 32
168	note: *	Rhodes Creek near Terry, MS mean= 3.220; sd= 0.269; skew= -0.133 area= 21.0sqmi; slope= 11.1ft/mi; length=	12.0mi	1948-69 1973	1,660 13	2,760 13	3,570 14	4,623 17	5,440 20	6,210 22	7,110 25	8,180 29

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station	station name and description		Period of							feet per mate (per	
				record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
	note: *	Strong River near Morton, MS mean= 3.321; sd= 0.245; skew= 0.118 area= 16.%sqmi; slope=10.7tt/mi; length=	5.6ml	1959-69 1974-75	1,970 15	3,120 15	3,970 17	5,000 20	5,810 23	6,550 25	7,440 28	8,460 32
	note: *	Strong River near Puckett, MS mean= 3.780; sd= 0.307; skew= 0.232 area= 248 sqmi; slope= 2.6ft/mi; length=	44.1mi	1955-88 1950	5,990 12	11,000 13	15,400 15	21,300 18	25,900 21	30,400 24	35,600 27	42,100 31
	note: *	Strong River at D'Lo, MS mean- 3.923; sd= 0.239; skew= 0.200 area- 425 sqmi; slope- 2.4ft/mi; length=	61.7mi	1929-88 1900	8,280 7	13,500 8	17,800 10	24,000 33	29,200 15	34,700 18	40,800 20	49,100 24
	note: *	Dabbs Creek near D'Lo, MS mean- 3.326; sd= 0.245; skew= 0.551 area- 57.2sqmi; slope= 4.5ft/mi; length=	29.3mi	1948-69 1980	2,060 12	3,520 14	4,830 16	6,710 20	8,190 23	9,640 26	11,300 29	13,300
	rote: *	Riles Creek near Mendenhall, MS mean= 3.310; sd= 0.341; skew= 0.222 area= 25.5sqmi; slope= 13.9ft/mi; length=	12.0mi	1949-50 1954-70 1974	1,930 17	3,560 17	4,810 19	6,290 22	7,410 24	8,430 27	9,710 30	11,100 34
	note: *	Boggans Ditch near Mendenhall, MS mean- 2.340; sd= 0.337; skew= -0.072 area- 0.91sqmi; slope= 71.1ft/mi; length=	1.3mi	1955-84	231 14	432 14	591 15	795 18	953 21	1,090 23	1,260 26	2,440 30
	note: *	Baking Powder Draw near Prentiss, MS nean- 2.053; sd= 0.467; skew= -0.316 area- 0.82sqmi; slope= 63.9ft/mi; length=	1.3mi	1955-77	159 20	339 18	486 19	67] 21	814 24	928 26	1,070 29	1,220 33
	note: *	Barrets Branch near Pinola, MS mean= 2.388; sd= 0.334; skew= 0.039 area= 0.88sqmi; slope= 47.1ft/mi; longth=	2.lmi	1955-77	240 16	429 16	569 17	732 20	860 23	964 25	1,100 28	0,240 32
	note: *	Big Creek near Finola, MS mean= 3.356; sd- 0.278; skew= 0.300 area= 45.9sqm; slope= 6.2ft/mi; length-	22.0mi	1948-69	2,210	3,860 15	5,200 16	6,990 20	8,380 23	9,720 25	11,300 28	13,100 32
178	note: *	Bradleys Ditch near Pinola, MS mean= 2.335; sd= 0.222; skew= -0.037 area= 0.54sqmi; slope= 27.5ft/mi; length=	2.7mi	1955-77 1983	207 11	309 11	377 12	459 15	522 18	578 20	647 23	726 27
	note: *	Copiah Creek near Hazlehurst, MS mean= 3.680; sd= 0.338; skcw= 0.284 area= 47.4sqmi; slope= 10.7ft/mi; length=	12.1mi	1955-88 1950,53	4,290 13	7,710 14	10,100 16	12,900 20	14,900 22	16,700 25)9,100 28	21,700 32
180	note: *fq	<pre>Pearl River at Rockport, MS g</pre>	242.Omi	1938-51 1985-88 1900,79,83 1874	31,800 11	47,400 13	58,700 15	73,900 20	85,90C 25	98,600 30	112,000 36	131,000 44

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	station no.	Station name and description	Period of record			Peak T-ye Standard					
				2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
181	note: *	Small Pinc Ditch near Monticollo, MS mean= 2.072; sd= 0.201; skew= -0.011 area= 0.16sqmi; slope= 126 ft/mi; length= 0.5mi	1955-77 1980-84	11 6 9	172 10	211 11	26: 14	299 16	335 19	377 21	424 25
182	note: *a	Pearl River near Monticello, MS mean= 4.547; sd= 0.183; skew= 0.100 area= 4,990 sqm1; slope= 1.0ft/m1; length= 273.0m1	1924-88 1900,02 1874	35,800 6	50,200 6	60,800 7	74,900 10	85,800 12	97,100 15	139,000	125,000
183	note: *	Roadside Park Ditch near Monticello, MS mean= 2.077; sd= 0.203; skew= 0.199 area= 0.25sgmi; slope= 103 ft/mi; length= 0.8mi	1955-77 1983	118 10	178 12	224 13	287 16	339 18	3 6 5 21	442 24	505 28
184	note: *	New Hebron Gulley at New Hebron, MS mean= 2.684; sd= 0.414; skew= 0.129 area= 2.50sqmi; slope= 44.7ft/mi; length= 2.2mi	1965-77 1957,83	493 21	952 20	1,290 21	1,650 23	1,940 26	2,170	2,480	2,790 35
185	note: *	Goines Draw near Prontiss, MS mean= 1.904; sd- 0.487; skew= 0.113 area= 0.34scmi; slope= 96.4ft/mi; length= 0.8mi	1955-84	99 19	224 19	324 20	44C 23	529 25	595 28	687 31	775 35
186	note: *	Plum Ditch near Prentiss, MS mean= 1.876; sd- 0.284; skew= 0.158 area= 0.23sgml; slope= 90.9ft/mi; length= 0.9mi	1955-76 1983	79 14	139 14	189 16	256 19	310 22	356 24	414 27	474 31
187	note: *	Whitesand Creek near Oak Vale, MS mean- 3.628; sd- 0.407; skow= 0.080 area- 130 sgmi; slope= 8.7ft/mi; length= 30.8mi		4,350 18	8,940 18	12,600 19	17,200 22	20,600 24	23,800 27	27,700 30	32,200 34
188	2489000 note: *g region 4	Fearl River near Columbia, MS mean= 4.564; sd= 0.174; skew= 0.150 area= 5,720 sqm1; slope= 1.0ft/m1; length= 326.0m1	1905-86 1874,1900	36,300 5	51,100	61,500	75,300 9	86,003 !!	97,100 .3	109,000	125,000 19
189	note: *	Elmors Draw near Columbia, MS mean- 2.571; sd= 0.269; skew= 0.165 area= 0.91sqmi; slope= 68.6ft/mi; longth= 1.4mi		355 11	585 12	754 13	959 17	1,120 19	1,260 22	1,430 25	1,630 29
190	2489160 note: * region 1	<pre>Kokomo Draw at Kokomo, MS moan= 2.560; sd- 0.317; skew= 0.185 area- 1.26sqml; slope= 42.2ft/ml; length= 1.5ml</pre>	1955-77 1983	353 15	629 15	835 17	1,080 20	1,270 23	1,420 25	1,620 28	1,840 32
191	note: *	Ten Mile Creek near Columbia, M5 mean= 3.298; sd= 0.419; skew= 0.114 area= 38.5sgmi; slope=16.8ft/mi; length= 13.8mi	1953-70	2,110 20	4,340 19	6,090 21	8,190 23	9,790 25	11,200 28	13,000 31	14,900 35
192	note: *	Lower Little Creek near Baxterville, MS mean- 3.492; sd= 0.395; skew= -0.079 area= 81.5sqmi; slope= 13.1ft/mi; length- 11.2mi	1961-70	3,810 23	7,690 21	10,800	14,600 24	17,500 26	20,100 28	23,300 31	26,700 35

Table 1.—Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	\$Lation hams and description	Period of record			Peak T-yo Standard					
			record	2-year	5-year	10-year	-	-	100-year	-	-
193	2489500 note: *g region 4	Pearl Rivor near Bogalusa, LA mean= 4.634; sd= 0.196; skew= 0.150 area= 6,570 sqm1; slope= 1.01t/m1; length= 338.	1938-88 Orai	42,500 7	62,600 8	77,200 9		113,000 16	129,000 19	147,000	
194	2490105 note: * region 1	Bogue Lusa Creek at Bogalusa, LA mean= 3.345; sd= 0.351; skew= 0.001 area= 72.7sgmi; slope= 9.6fc/mi; length= 21.	1964-85 4mi	2,450 16	4,900 16	6,980 18	9,820 20	12,000 23	14,200 26	16,600 28	19,500 32
195	2490250 note: * region l	Bogue Chitto near Brookhaven, MS mean= 3.380; sd= 0.346; skcw= 0.074 area= 28.3sgmi; slope= 6.3ft/mi; length= 10.	1953-70 6m1	2,260 18	4,050 18	5,330 19	6,810 22	7,900 24	8,920 27	10,100 30	11,600 34
196	2490300 note: * region l	<pre>Big Crock at Bogue Chitto, MS mean= 3.532; sd= 0.331; skew= -0.062 area= 55.1sqmi; slope= 5.8ft/mi; length= 16.</pre>	1952-70 2mi	3,300 16	5,900 16	7,820 17	10,100	11,900 23	13,500 25	15,400 28	17,600 32
197	pote: *	Bogue Chitto near Tylortown, MS mcan= 4.120; sd= 0.349; skew= -0.313 area= 492 sqm; slope= 3.3ft/m1; longth= 59.	1945-88 1936 2mi		24,700 12	33,300 12	43,900 15	51,600 18	59,100 20	67,400 23	77,700 27
198	2490550 note: * region l	Middle Fork Hickory Fiat near Tylertown, MS mean= 2.694; sd= 0.258; skew= 0.160 area= 1.46sgmi; slope= 38.3ft/mi; length= 2.	1953-84 4mi	466 11	746 12	944 14	1,180 17	1,370 19	1,530 22	1,730 25	1,950 29
199	2490700 note: *	Union Creek near Tylertown, MS mean= 2.934; sd= 0.429; skew= 0.083 area= 12.4sgmi; slope=16.8ft/mi; length= 6.	1953-69 1900	985	2,040 20	2,880 21	3,870 23	4,630 25	5,270 28	6,080 31	6,960 35
200	2490750 note: * region 1	McGees Croek at Tylortown, MS mean= 3.617; sd= 0.353; skew= -0.052 area= 152 sgml; slope= 5.3ft/mi; length= 32.	1952-74 1980,83 8mi 1900,43		8,500 15	11,900 J6	16,400 19	19,900 22	23,300 24	27,200 27	31,800 31
201	note: *	Bogue Chitto at Franklinton, LA mean= 4.307; sd= 0.322; skew= -0.281 area= 985 sgmi; slope= 4.4ft/mi; length= 65.	1922-88 1900 3mi	23,880 9	37,500 9	50,000 9	65,500 10	78,200 11	92,200 12	99,800 13	122,000
202	2492000 note: * region 4	Bogue Chitto near Bush, LA mean= 4.267; sd= 0.307; skew= -0.154 area- 1,210 sqm; slope+ 4.0ft/m1; length- 92.	1938-88 4mi	19,100 10	34,000 10	45,700 10	60,500 11	72,900 12	86,300 13		113,000 15
203	2492350 note: * region 1	East Hobolochitto Creek at Picayune, MS mean= 3.618; sd= 0.188; skew= 0.596 area= 114 sqm; slope= 5.7ft/m; longth= 32.	1957-66 1969-71 7mi 1916,28,	4,000 11 74	6,160 !3	8,110 15	11,000 19	13,500 22	15,900 25	18,600 28	22,00C 32
204	note: *	West Hobolochitto Creek near McNoill, MS mean= 3.784; sd= 0.253; skew= 0.053 area= 175 sgmi; slope= 5.2fv/mi; length= 33.		6,010 12	9,970 13	13,100 14	17,300 17	20,700	24,100 23	27,900 25	32,600 29

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station nc.	Station name and description		Period of record	botto	m line	Standard	error cf	T-year f	e (cubic lood esti:	nate (per	cent)
				record	2-year	5-year	10-year	25-year	-	100-year	-	
205	2492600 note: gh region 4	Pearl River at Pearl River, LA mean= 4.757; sd- 0.221; skew= 0.150 area- 8,590 sqn1; slope- ; Jength-		1900-88 1874	56,500 5	87,400 6	111,000	143,000	169,000 12		228,000	
206	3592718 note: * region 1	Little Yellow Creek near Burnsville, MS mean= 3.217; sd- 0.343; skew= -0.093 area= 24.7sqmi; slope= 13.9ft/mi; length-	- 7.4mi	1974-88	1,790 18	3,370 18	4,620 19	6,170 21	7,370 24	8,440 26	9,750 29	11,200 33
207	note: *	Yellow Creek near Doskie, MS mean= 3.675; sd= 0.301; skew= 0.004 area= 143 sqm1; slope= 5.5ft/m1; length-	- 16.8mi	1938-61 1973-78 1902	4,930 12	9,010 13	12,430 14	17,100 17	20,900	24,500 23	28,700 25	33,700 29
	note: *	Chambers Creek near Kendrick, MS mean= 3.350; sd= 0.368; skew= -0.220 area= 21.1sqm1; slope- 11.8ft/mi; Jength-			2,140 17	3,910 16	5,160 17	6,570 20	7,580 22	8,480 25	9,590 28	10,800 32
209	note: *	Pool Branch near Ripley, MS mean= 2.499; sd= 0.175; skew= -0.089 area= 1.24sqmi; slope= 36.0ft/mi; length=		1965-77	31.6 1.2	453 12	553 13	686 16	796 19	897 21	1,020	1,160 28
210	7029270 note: * region 1	Hatchie kiver near Walnut, M5 mean= 3.849; sd= 0.271; skew= -0.058 area= 272 sqn1; slope= 4.4ft/m1; length=	32.4mi	1947-80	7,230 11	12,500 11	16,600 13	22,300 15	27,000 18	31,700 20	36,900 23	43,600 27
	7029300 note: *	Tuscumbia River Canal near Corinth, MS mean= 3.930; sd= 0.260; skew= 0.046 area= 278 sqml; slope= 3.9ft/m1; length-		1950-80	8,550 11	14,500 12	19,200 13	25,800 16	31,300 19	36,300 21	42,100 24	49,600 28
21.2	noLe: *	Hatchic River at Pocahontas, TN mean= 4.175; sd- 0.250; skew= 0.071 area= 837 sgmi; slope= 2.5ft/mi; length-			15,300 10	25,900 10	35,300 11	49,400 12	63,200 12	79,100 13	88,600 14	114,000 16
	note: c	Eurricane Crook near Walnut, MS mean= 3.156; sd= 3.051; skew= -0.351 area= 20.2sqmi; slope= 17.1ft/mi; length=		1953-70	1,440 3	1,590 3	1,690 3	1,780 4		1,930 6	2,000 7	2,090 9
214	7030365 note: c region l	Wesley Branch near Walnut, MS mean= 2.322; sd= 0.26]; skew= 0.093 area= 2.17sqmi; slope= 63.5ft/mi; length-	- 2.4mi	1966-77	256 17	476 17	680 18	971 21	1,210 24	1,420 26	1,680 29	1,950 33
	note: *	<pre>Rolf River at Rossville, TN mean= 3.966; sd= 0.285; skew= -0.480 area= 503 sqmi; slope= 3.0ft/mi; length=</pre>	: 58.9mi	1930-71	9,850 11	16,600 10	21,430 10	27,700 13	32,700 15	37,600 18	43,100 20	49,900 24
	note: *	Cane Creek near New Albany, MS mean= 3.478; sd= 0.213; skew= -0.107 area= 22.2sqm; slope= 12.8ft/m; length-		3939-43 3950-74	3,020 10	4,540 10	5,570 11	6,920 14	7,890 16	8,990 19	9 ,9 20 21	11,300 25

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map no.	Station	\$tation name and description		Period of			Peak T-ye Standard					
				record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
217	note: *ab	Hell Creek near New Albany, MS og mean= 3.624; sd= 0.155; skew= -0.055 area= 26.8sqm1; slope= 8.6ft/w1; length-	12.7աi	1939-42 1952-88	4,220 9	5,680 10	6,630 11	7,800 15	8,660 18	9,510 22	10,400 26	11,500 31
	note: *	Cracker Ditch near Pontotoc, MS mean= 2.076; sd= 0.157; skew= -0.113 area= 0.23sqmi; slope- 91.6ft/mt; longth=	0.8mi	1955-58 1962-75	124 9	169 9	198 10	239 13	270 15	303 18	330 20	37 <i>2</i> 24
	note: *	Little Tallahatchie River at Etta, MS mean= 4.439; sd= 0.229; skew= -0.152 area= 526 sqmi; slope= 5.3ft/mi; length=		1937-88	27,800 8	43,100 8	54,000 9	68,600 ll	79,800 13	92,200 16	104,000	120,000
	note: *	Fice Creek at Elta, MS mean- 3.228; sd= 0.374; skew= 0.052 area- 8.78sqmi; slope= 15.1ft/mi; length=	6.2mi	1952-70	1,620 18	2,910 19	3,740 20	4,620 24	5,070 26	5,700 28	6,100 31	6,78G 34
	note: ac	Cypress Crock near Etta, MS mear= 3.675; sd= 0.254; skew= -0.233 area= 28.5sqmi; slope= 9.4ft/mi; length=	8.8mi	1939-42 1952-88	4,850 11	7,790 11	9,870 13	12,600 16	14,600	16,700 24	18,800 28	21,700 34
	note: *ab	North Tippah Crock near Ripley, MS g mean- 3.602; sd= 0.165; skew= -0.190 area- 19.3sqmi; slope= 16.1ft/m1; length=	7.7mi	1939-42 1952-80 1983-88,19	4,050 9 48	5,530 10	6,460 11	7,590 14	8,400 17	9,190 21	9,960 25	11,000 30
	note: *	Tippah Creek mear Polts Camp, MS mean- 4.038; sd= 0.192; skew= -0.239 area- 355 sqmi; slope= 3.4ft/mi; length=	43.4mi	1943-83	11,400 10	16,600 10	20,300 11	25,600 14	29,900 16	35,000 19	39,830 21	46,500 25
	note: *	Clear Creek near Oxford, MS mean= 3.466; sd= 0.195; skew= -0.153 area= 10.4sqmi; slope= 25.4ft/mi; length=	4.2mi	1939-41 1950-74	2,910 9	4,230 9	5,060 10	6,110 13	6,840 15	7,620 17	8,280 20	9,240 23
	note: ak	Little Tallahatchie River at Sardis Dam, MS mean= 3.692; sd+; skewm area= 1,540 sqmi; slope= 2.9ft/m1; length+	1mE.07	1940-83		==					==	
	rote: *ab	Tallahatchie River near Sardis, MS k mean= 3.825; sd=; skew= area= 1,600 sqmi; slope= 2.7ft/mi; length=	77.9mi	1929-60								
	note: ad	Little Tallahatchie River(Panola-Quitman Floo near Batesville, MS moan- 4.098; sd- 0.169; skow= 0.400 arca-1,770 sqmi; slope= 2.3ft/mi; length=		1942-63 1937,40	12,200 10	17,300 12	21,000 15	26,10C 2G	30,300 26	34,800 32	39,600 38	46,600 48
	note: *	Yocona River near Oxford, MS mean= 3.987; sd= 0.289; skew= 0.111 area= 254 sqmi; slope= 4.1ft/m2; Length=	34.8mi	1947-88	9,800 11	17,300 11	23,400 13	32,000 17	38,700 19	46,600 22	53,500 25	63,200 28

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description		Perind of record			Peak T-ye Standard					
		<u> </u>		ecord	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-yea -
229	7274250 note: * region 2	Otoucalofa Creek at Water Valley, MS mean= 3.643; sd= 0.239; skew= 0.251 area= 04.lsqmi; slope= 7.9ft/ml; longth=	19.1mi	1952-88	4,470 10	7,340 11	9,740 13	13,400 16	16,300 19	19,700 22	22,700 25	27,100 28
230	note: *ab	Yocona River at Enid Dam near Enid, MS ok mean- 3.384; sd; skow area- 606 sqmi; slope= 3.2ft/mi; length=	63.5mi	1927-83	 					==	==	
231	note: *	Long Creek at Courtland, MS mean= 3.932; sd= 0.226; skew= 0.075 area= 62.3sqmi; slope= 13.8ft/m1; length=	17.6mi	1940-43 1952-88 1948	8,410 9	13,000 10	16,400 12	20,900 15	24,000 17	27,700 20	30,700 22	35,200 26
232	nole: *	Coldwater River near Lewisburg, MS mean= 3.995; sd= 0.295; skew0.570 area= 213 sqmi; slope= 4.2ft/mi; length=	49.9mi	1940-58	10,400 15	17,300	21,900	27,200 18	30,700 20	35,300 23	38,900 26	44,100 30
233	note: ag	Pigeon Roost Crock near Lewisburg, MS mean= 3.882; sd- 0.232; skow= 0.360 arca= 229 sqm; slope= 8.7tt/mi; length=		1940-58	7,380 13	11,800 16	15,400 20	20,700 28	25,300 35	30,400 43	36,200 52	45,000 66
234	nole: *	Coldwater River near Coldwater, MS mean= 4.243; sd= 0.355; skew= -0.249 area= 634 sqmi; slope= 3.2ft/mi; length=		1929 42	19,300 20	35,700 19	48,00G 2G	63, 900 23	74,600 25	88,830 28	99,500 31	115,000 34
235	role: *	James Wolf Creek tributary near Looxahoma, MS mean- 2.345; sd= 0.245; skew0.309 area- 0.29sgmi; slope= 149 ft/mi; length=		1965-77	242 16	372 15	455 16	563 19	630 22	701 25	743 27	823 31
236	note: *	Hickahala Creek noar Senatobia, MS mean= 3.9//; sd= 0.226; skow==0.027 area= 122 sgmi; slope= 9.9ft/mi; length=	19.6mi	1943-58	9,650 13	15,100 14	19,200 15	24,800 18	29,000 21	34,000 24	38,000 27	43,8C0 30
237	note: *	Senatobia Creok near Senatobia, MS moan- 4.155; sd= 0.096; skew= -0.330 area= 82.0sgmi; slope=10.3ft/mi; length-		1943-58	14,100 6	17,000	18,600 6	20,500 8	21,700 30	23,000	24,300	26,000 16
238	note: ak	Coldwater River at Arkabutla Dam, MS mean= 3.659; sd; skew= area= 1,000 scm; slope= Z.9tt/mi; length=	00.6mi	1938-83							==	
239	7279300 note: afo region 4	Coldwater River at Prichard, MS g mean= 3.772; sd= 0.067; skew= -0.450 area= 1,210 sqmi; slope= ; length=		1946-58	5,980 5	6,750 4	7,140 5	7,550 6	7,810 8	8,040 9	8,240 11	8,480 13
240	note: *bi	Coldwater River at Savage, MS i mean= 4.072; sd= 0.377; skew= -0.262 area= 1,230 sgmi; slope= 2.8ft/mi; length=	94.9mi	1909-12 1936-42			 	 	 	 		

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station name and description no.	Period of record				Peak T year flood magnitude (cubic feet per seco Standard error of T-year flood estimate (percent					
		10070	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year	
					<u>-</u>						
241	7279600 Arkabutla Crock near Arkabutla, MS note: * mean= 4.045; sd= 0.115; skew= -0.342 region 2 area= 98.1sqmi; slope= 7.5ft/mi; longth= 21.0mi	1947-58	11,000 8		15,400 9			20,400 15	21,900 17	24,100 20	
242	7279970 Bobo Bayou at Bobo, MS note: gh	1946-58	1,710 3	1,830 2	1,890 3	1,940 3	1,970 5	2,000 6	2,020 7	2,040 8	
243	7280000 Tallahatchie River near Lambert, MS note: ag	1936-83	11,200 4	13,300 4	14,400	15,700 5	16,500 6	17,300 7	18,000 9	18,900 10	
244	7280270 Tillauoba Creek below Oakland, MS note: * moan= 3.725; sd- 0.163; skew= -0.190 region 2 area= 37.1sqmi; slope= 11.7ft/mi; length= 12.3mi	1975-84	5,210 12	7,180 12	8,430 13	10,000 16			13,700 24		
245	7280340 South Fork Tillatoba Crock mear Charleston, MS note: * mean= 3.784; sd= 0.219; skew= -0.208 region 2 area= 53.9sqmi; slope= 9.7ft/ml; length= 15.2mi		6,020 14	9,090 14	11,100 15	13,700 18	15,500 21	17,600 23	19,300 26		
246	7281000 Tallahatchie River at Swam LaKe, MS note: ag mean= 4.258; sd- 0.131; skew= 0.300 region 3 area= 5,130 sqmi; slope= 1.8ft/mi; length= 133.6mi	1930-83	17,900 6	23,300 7	26,900 8	31,/00 11	35,300 14	39,100 17	42,900 21	48,300 25	
247	7282000 Yalobusha River at Calhour City, MS note: *abg mean= 4.393; sd- 0.258; skew= -0.273 region 2 area= 305 sqmi; slope= 3.0ft/mi; length= 30.2mi	1949-88	25,400 14	41,000 14	52,000 16	66,100 20			98,300 36		
248	7282300 Sabougla Crock tributary at Sabougla, MS note: * mean= 2.240; sd= 0.178; skew= 0.102 region 2 area= 0.50sqmi; slope= 38.2ft/mi; length= 1.3mi	1967-77	180 12	258 13	314 15	395 19	454 21	519 24	568 27	647 30	
249	7287500 Yalobusha River at Graysport, MS note: * mean= 6.259; sd= 0.275; skew= =0.066 region 2 area= 607 sqmi; slope= 2.3ft/mi; length= 56.9mi		18,300 18	30,900 18	40,400 20	53,100 23		74,400 28	83,900 31	97,300 34	
	7283000 Skuna River at Bruce, MS note: *abq mean= 4.252; sd= 0.255; skew= -0.277 region 2 area= 254 sqmi; slope= 3.6ft/mi; length= 31.2mi		18,300 13	1.3	37,100 15	47,100 19		62,000 28	69,500 34		
251	728349C Caney Creek near Coffeeville, Ms note: * mean= 2.942; sd= 0.139; skew= -0.246 region 2 area= 1.97sqmi; slope= 33.5ft/mi; length= 2.8mi	1955-84	876 7	1,140	1,290 7	1,470 9	1,600 11	1,720 13	1,840 15	1,990 18	
2 52	7283500 Skuna River near Coffeeville, MS note: *bi mean= 4.182; sd= 0.208; skew= 0.218 region 2 area= 435 sgmi; slope= 2.9ft/mi; length= 52.0mi	1940-49									

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	station no.		Period of record	top line- Peak T year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)								
	_		1000.4	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year	
253	note: ak	Yalobusha River at Grenada Dam near Grenada, MS mean= 3.620; sd=; skew area= 1,320 sqmi; slope= 2.2ft/ml; length= 66.1ml	1954-83			 			 	 	==	
254	note: *	Tie Plant Branch near Grenada, MS mean= 1.967; sd= 0.153; skew= 0.028 area= 0.17sqmi; slope- 93.7ft/mi; length= 0.6mi	1966-77 1955	97 10	133 11	158 12	194 15	221 18	251 21	275 23	313 27	
255	note: *ab	Yalobusha River at Grenada, MS weg mean= 4.036; sd= 0.147; skew= 0.950 area= 1,550 sqm1; slope= 2.2ft/mi; length= 69.3mi	1909-11 1927-58	10,300 18	14,000 22		 					
256	note: *	Long Creek near Cascilla, MS mean= 2.955; sd= 0.166; skcw= 0.007 area= 1.64sqm1; slope= 45.1ft/mi; length= 1.5mi		886 8	1,220 9	1,430 10	1,700 13	1,890 15	2,080 18	2,240 20	2,480 23	
257	note: c	mean= 2.844; sd= 0.169; skew= 0.096	1965-77	702 11	988 12	1,180	1,450 17	1,640	1,850 22	2,000 25	2,260 29	
258	note: qh	mean= 1.256; sd= 0.115; skew= 0.273	1967-77	9 18	22 10	26 12	29 17	3 <i>2</i> 21	35 26	38 31	42 38	
259	note: acq	Yalobusha River at Whaley, MS 		8,510 11	10,500 14	11,900 18						
260	note: *	Thompson Creek at McCarley, MS mean= 3.399; sd= 0.102; skew- 0.097 area 14.4sqmi; slope= 19.5ft/mi; length- 8.4mi	1950-66	2,500 6	3,080 7	3,470 8	3,990 10	4,390 12	4,850 14	5,270 17	5,890 20	
261	rc.o. *	Big Sand Creck trib. near North Carrollton, MS mean= 1.674; sd- 0.081; skew=-0.122 area= 0.06sqmi; slope= 106 ft/mi; length= 0.5mi	1965-84	4 8 4	56 5	61 5	66 7	71 8	75 10	79 11	85 14	
262	note: *	Big Sand Creek at Carrollton, MS mean= 3.960; sd= 0.256; skew0.174 area= 74.1sqmi; slope- 10.6ft/mi; length= 13.6mi		8,910 14	14,200 14	17,900 15	22,400 18	25,300 21	28,900 24	31,500 26	35,500 30	
263		Big Sand Creek at Valley Hill, MS mean= 4.254; sd+ 0.142; skew= 0.012 area= 110 sqmi; slope= 6.8ft/mi; length= 23.0mi		16,700 10	21,900 10	24,900 12	28,400 15	30,600 17	33,600 20	36,000 23	39,600 26	
264	note: aq	Yazoo River at Greenwood, MS	1908-12 1928-83	22,900 5		32,100 7	36,433 9	39,50 0 11	42,600 13	45,600 15	49,500 19	

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	station no.		Period of record	top line Poak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)								
			record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year	
265	7287050 note: c region Z	Palucia Creek tributary near Carrollton, MS mean= 1.841; sd= 0.253; skew= 0.242 area= 0.43sqn1; slope= 75.2ft/mi; length= 1.7m1	1964-77	89 15	158 16	220 19	319 22	391 25	470 28	525 30	613 34	
266	note: *	Mosquito Lake tributary no.1 at Itta Bena, MS mean= 1.754; sd= 0.126; skew= 0.338 area= 0.11sqmi; slope= 10.6ft/mi; length= 0.3mi]966-84	56 8	73 9	8 4 1. 1.	101 15	113 18	127 21	141 24	159 28	
267	7287170 note: * region 3	Mosquito Lake tributary no.2 at Itta Bena, MS mean= 1.845; sd= 0.095; skow= 0.168 area= 0.13sqmi; slope= 10.6ft/mi; length= 0.6m1	1966-84	70.0 6	84.0	94.0 8	106 10	115 13	125 15	135 17	149 21	
268	7287350 note: *	Fannesgusha Creek near Tchula, MS mean= 3.976; sd= 0.232; skcw= 0.124 area= 100 sgmi; slope= 6.9ft/mi; length= 28.2mi	1421-88	9,180 9	14,400	18,000 12	22,700 15	25,900 17	29,800 20	33,100 23	37,800 26	
269	7287480 note: * region 2	Piney Crock near Yazoo City, MS mean= 3.86:; sd= 0.236; skew0.355 arca= 70.3sqmi; slope- 9.0ft/mi; length= 20.4mi	1953-70 1951	7,290 13	11,200 12	13,700 13	16,800 16	18,800 18	21,300 21	23,200 24	26,100 28	
270	7287505 note: gh region 3	Broad Lake tributary no.1 near Yazoo City, MS mean= 1.278; sd= 0.102; skew= -0.171 area= 0.11sqmi; slope=10.6ft/mi; length= 0.4mi	1966-77	19 8	23 8	26 9	28 12	30 14	32 17	33 20	36 24	
	7287523	Short Creek tributary near Yazoo City, Ms mean- 2.880; sd= 0.155; skow0.426 area- 1.49sqmi; slope-48.7ft/mi; length= 2.5mi	1964-73	756		1,140	1,310	1,410 17	1,530	1,620 22		
	7288500 note: * region 3	Big Sunflower River at Sunflower, MS mean 3.802; sdr 0.164; stew -0.155 area 767 sqmi; slope 0.511/mi; length= 103.5mi	1936-83	6,310 6	6	10,000	9	16	12	14,900 14	16,200 17	
273	7288568 note: c region 3	Quiver River tributary near Schlater, MS mean= 1.516; sd= 0.065; skew= 0.022 area= 0.18sqmi; slope= 10.0ft/mi; length= 0.5mi	1967-79	3 1 5	38 5	4 <i>2</i> 6	46 8	50 10	54 12	58 13	64 16	
274	7288570 note: * region 3	Cuiver River near Coddsville, MS mean= 3.420; sd= 0.186; skew= 0.128 area= 292 sgmi; slope= 0.7ft/ml; length= 55.2mi	1938-60	2,620 9	3,760 10	4,550 12	5,560 16	6,310 19	7,020 22	7,650 25	8,510 28	
275		Bogue Phalia near Leland, MS mean= 3.775; sd= 0.131; skew= -0.412 area- 484 sqm1; slope= 0.8ft/m1; length- 61.8m1		5,910 9	7,580 8	8,500 9		10,200 14	10,800 16	11,400 19	12,200 22	
276	note: qh	Big Sunflower River at Little Callao Landing, MS mean- 4.176; sd- 0.086; skow= -0.140 area= 2,290 sqmi; slope= 0.4ft/mi; length= 157.0mi	1948-58			19,300 8		22,200 12	23,300 15	24,430 17	25,700 21	

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description		Period of	<pre>Lop line Peak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)</pre>								
				record	2-year	5-year)0-year	25-year	50-year	100-year	200-year	500-year	
277	note: *	Doez Creek noar Hollandale, MS mean= 2.757; sd- 0.125; skow0.017 aroa= 98.0sgmi; slope= 0.4lt/ml; lengtn-	69.8mi	1946-58	585 8	745 9	845 10	970 13	1,070 16	1,160 19	1,250 21	1,360 25	
278	7289000 note: gh.	Mississippi River at Vicksburg, MS mean= 6.132; sd= 0.134; skcw0.583 area= 1,140,400sqmi; slope; length-		1927-86 1897-99 1900-22 1858,85	1,390 3	1,660 2	1,810 3	1,960 3	2,050 4	2,130 5	2,200 6	2,290 8	
279	note: *	Durden Creek at Vicksburg, MS mean= 3.213; sd= 0.217; skew= 0.090 area= 5.50scmi; slope= 17.6ft/mi; length=	5.4mi	1941-46 1953-58 1935,49	1,520 13	2,240 14	2,670 16	3,160 20	3,440 22	3,820 25	4,070 28	4,510 31	
280	note: *	Big Black River tributary near Eupora, MS mean- 2.86]; sd= 0.160; skew= -0.116 area- 2.29sqmi; slope= 27.8ft/mi; length=	2.8mi	1965-77	726 9	953 10	1,100	1,290 14	1,420 16	1,570 18	1,700 21	1,890 24	
281	note: *f	Big Black River near Kilmichael, MS mean= 4.183; sd= 0.252; skew= -0.160 area= 564 sqmi; slope= 3.5ft/mi; length=		1937-58 1962,73 1979,83	16,200 12	26,70C 12	34,820 14	46,700 17	56,400 19	67,900 22	78,100 25	92,700 28	
282	note: *	Downing Branch near French Camp, MS mean= 2.693; sd= 0.107; skew= -0.236 area= 1.74sqmi; slope= 24.5ft/mi; length=	2.5mi	1965-77	498 7	612 7	681 8	769 10	836 12	9C9 14	973 16	1,070 19	
283	note: *	<pre>Hays Creek tributary no.1 near Valden, MS mean= 3.277; sd= 0.207; skew= -0.205 area= 14.6sqmi; slope= 15.5ft/mi; length=</pre>	6.Cmi	1960-88	1,950 9	2,900 9	3,550 11	4,420 13	5,080 15	5,820 18	6,460 20	7,390 24	
284	note: *	Hurricane Creek tributary near Vaiden, MS mean= 2.540; sd+ 0.150; skow= 0.051 area= 0.40sqmi; slope= 71.8ft/mi; length+	1.Cmi	1966-77	337 10	448 11	511 13	596 16	646 1.8	704 21	745 24	816 27	
285	note: *	<pre>Zilpha Creek near Kosciusko, MS mcan= 3.633; sd= 0.349; skew= 0.124 area= 86.6sqmi; slope= 6.8ft/mj; length=</pre>		1953-70 1979	4,710 17	9,010	12,500 20	17,100 23	20,200	24,100	26,900 31	31,000 34	
286	note: *	Big Black River at West, MS mean= 4.357; sd= 0.230; skew= -0.047 area= 1,030 sqmi; slope= 2.3ft/mi; length-	76.8mi	1937-88 1927,30	22, 100 7	34,500 8	43,40C 8	55,000 10	65,400 1:	77,500 12	84,800 13	104,000	
287	note: *	Sharkey Creek tributary near West, MS mean= 2.214; sd= 0.137; skew0.007 area= 0.30sqmi; slope= 50.6ft/mi; length=	1.0mi	1967-79 1982-84	164 8	215 9	247 13	290 13	320 15	354 18	382 20	425 23	
288	note: *	Tacketts Creek tributary near Pickens, MS mean= 2.146; sd= 0.164; skw= -0.417 area= 0.15sqmi; slope= 110 ft/mi; length-	0.5mi	1965-84	144 9	193 9	223 10	258 12	282 14	306 17	325 19	354 22	

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

May	Station no.			top lina Peak T-year flood magnitude (cubic feet per second) bottom line Standard error of T-year flood estimate (percent)							
			record	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year
289	7289500 note: *f region 4	Big Black River at Pickens, MS mean= 4.256; sd= 0.323; skew= -0.257 area= 1,490 sqmi; slope= 1.7ft/mi; length= 120.0mi	1937-73 1979,83 1892,1927,3	18,600 11 c	33,000 11	43,800 11	56,800 12	68,100 12	80,800 13	86,600 14	107,300 16
290		Big Cypress Creek near Vaughn, MS mcan- 3.843; sd= 0.161; skew= 0.229	1960-70	6,520 11	8,940 13	10,500 15	12,600 18	14,000 21	16,000 24	17,700 27	20,100 30
	note: * region 2	area 86.6sqmi; slope 3.4ft/mi; length 28.7mi	1979	11	13	15	10	21	2.1	2.	30
291	note: *	Doaks Creek near Canton, MS mean= 3.795; sd= 0.254; skew= 0.283 area= 164 sqmi; slope= 4.4ft/mi; length= 31.4mi	1948-70 1973 1979	6,350 12	10,700 13	14,500 16	19,900 20	24,100	29,200 25	33,400 28	39,500 32
292		Boar Creek near Madison, MS	1948-58	1,820	2,890	3,770	5,070	6,050	7,210	8,120	9,460
	note: * region 2	mean= 3.238; sd= 0.218; skew= 0.237 area= 24.4sqmi; slope= 7.5ft/mi; length= 9.1mi	1979	15	16	18	2.2	25	27	30	33
293		Tilda Bogue near Canton, MS	1948-88	2,710	4,460	5,680	7,270	8,400	9,650	10,700	12,200
	note: * region 2	mean- 3.417; sd= 0.271; skew= -0.280 area- 24.8sqmi; slope= 10.9ft/mi; length= 9.4mi		10	10	11	14	16	19	21	25
294		Bachelor Creek at Canton, MS	1953-70	734	972	1,140	1,360	1,530	1,710	1,890	2,150
	note: dg region 2	mean= 2.874; sd= 0.139; skew= 0.350 area= 3.85sqmi; slope= 14.7ft/mi; length= 3.0mi	1979 1973,75	7	8	10	1,4	18	22	26	32
295		Panther Crock near Flora, MS	1965-77	1.86	222	242	266	284	303	31.9	343
	note: * region 2	mean= 2.267; sd= 0.091; skew= -0.261 area= 0.26sqmi; slope= 67.9ft/mi; length= 0.7mi	1979	6	6	7	8	10	12	1.4	16
296		Panther Creek tributary near Flora, MS	1964-85	99	125	139	154	166	176	185	198
	note: * region 2	mean= 1.984; sd= 0.129; skew= -0.570 area= 0.07sgmi; slope- 192 ft/mi; length= 0.3mi		7	6	6	8	10	12	15	17
297		Big Black River near Bentonia, MS	1929-58	26,800	41,500	52,130	65,000	76,900 12	90,400	96,300 14	118,000
	note: *f region 4	mean= 4.439; sd= 0.232; skcw= -0.076 area= 2,340 sqmi; slope= 1.3ft/mi; length= 172.0mi	1962 1973,79,83	9	9	10	11	2.2	13	1.4	1.5
298		Boque Chitto near Flora, MS	1953-70	5,590	10,200	13,700	18,300	21,500	25,400	28,400	32,700
	note: * region 2	mean- 3.690; sd+ 0.356; skew= -0.411 area= 126 sqmi; slope= 4.2ft/mi; longth= 22.0mi	1979,80	17	1.6	17	23	23	26	28	32
299		Big Black River near Bovina, MS	1936-88	24,600	39,400	50,500	64,500	76,800	90,200		117,000
	note: * region 4	mean= 4.391; sd= 0.244; skew= -0.017 area= 2,770 sqmi; slope= 1.3ft/mi; length= 216.0mi		8	8	9	10	11	12	1.3	15
300		Clear Crock near Bovina, MS	1953-88	5,560	9,240	11,900	15,200	17,300	19,800	21,700	24,400
	note: *	mean= 3.763; sd= 0.269; skew= 0.172 area= 32.0sqmi; slope-16.6ft/mi; longth= 9.3mi		11	12	14	17	20	23	26	29

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

	Station no.	Station name and description	0			o line Peak T-year flood magnitudo (cubic foot per second n line Standard error ot T-year flood estimate (percent)							
			rec	cord	2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-year	
301	note: *	Flootwood Creek near Bolton, MS mean- 3.312; sd= 0.273; skew= -0.349 area= 13.0symi; slope- 15.1ft/mi; length-	196 6.5mi 1979		2,110 18	3,370 17	4,210 18	5,230 21	5,870 23	6,660 26	7,200 29	8,080 32	
302	note: *	Unnamed Creek near Bolton, MS mean= 2.863; sd= 0.236; skew= -0.368 area= 3.10sqmi; slope= 26.0ft/mi; length=	196 2.7ml 1979	50 - 70	773 15	1,180 15	1,440 16	1,790 19	2,020 21	2,280 24	2,480 27	2,790 30	
303	note: *	Dry Draw near Brookhaven, MS moan= 2.225; sd= 0.190; skew= 0.340 area= 0.20sqmi; slope= 100 ft/mj; Length=		56-77 5	166 12	240 14	291 16	356 20	396 23	440 26	467 29	519 32	
304	note: *	Bayou Pierre near Carpenter, MS mean= 4.223; sd= 0.214; skew= -0.598 area= 375 sqm; slope= 4.3ft/mi; length=	194	10,75	17,500 9	25,600 8	30,600 9	36,600 11	41,000 14	46,200 16	50,700 19	57,000 22	
305	note: *	Whiteoak Creek tributary near Utica, MS mean= 2.771; sd= 0.145; skew= 0.069 area= 1.36sqmi; slope= 25.0ft/mi; length=	196 2.5mi	55-84	575 8	761 9	877 10	1,020 13	1,110	1,220 38	1,310 20	1,440 23	
306	note: *	Bayou Ricrre near Willows, MS moan- 4.413; sd= 0.216; skew= 0.174 area- 654 somi; slope= 3.9ft/mi; length=	195 70.7mi	59-88 2	25,500 9	39,400 10	50,000 12	64,900 16	76,300 18	90,300 21	103,000	120,000 27	
307	note: *	Clarks Crock near Pattison, MS mean= 3.951; sd= 0.231; skew= 0.264 area= 75.0scmi; slope= 9.5ft/mi; length=		52-88	8,500 11	13,200 12	16,700	20,900 18	23,700 20	27,200 23	29,900 26	34,000 30	
308	note: *	Little Creek noar Fayette, MS mean- 2.897; sq= 0.179; skew= -0.099 area- 1.71sqmi; slope= 54.8ft/mi; length-		67-88	792 9	1,120	1,330 11	1,600 13	1,790 16	1,990 18	2,160 21	2,400 24	
309	note: *	Coles Creck near Fayette, MS mean= 4.508; sd= 0.175; skew= 0.027 area= 260 sqmi; slope= 7.3ft/mi; length=		51,-88 3		43,300 9	51,300 10	61,000 13	67,600 35	75,400 18	82,100 20	91,700 23	
310	note: c	St. Catherine Creek near Natchoz, MS mean= 4.036; sd= 0.324; skcw= -0.074 area= 54.3sqmi; slope= 12.4ft/mi; length=	195 15.7m1	50-60	8,890 21	14,400 21	17,400 22	20,400 25	21,900 27	24,500 30	25,900 32	28,700 35	
311	note: da	Spanish Bayou at Natchez, MS mean= 3.060; sd= 0.144; skew0.087 area- 2.46sqmi; slope= 27.9ft/mi; length=			1,150 10	1,520 11	1,750 13	2,030 17	2,240 20	2,430 24	2,630 29	2,880 35	
312	note: *	Homochitto River at Eddiceton, MS mean= 4.215; sd= 0.237; skew0.309 area= 101 scmi; slope= 6.21t/mi; length-	193 32.6mi	39-88 1	.6,500 8	25,400 8	31,400 9	38,500 11	43,300	48,500 15	53,100 18	59,300 21	

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Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

Map no.	station no.	Stallen name and description		Period of record	top line Peak T-year flood magnitude (subic feet per second) bottom line Standard error of T-year flood estimate (percent)								
					2-year	5-year	10-year	25-year	50-year	100-year	200-year	500-yea <i>r</i>	
313	7291250 note: * region 2	McCall Creek near Lucion, MS mean= 3.897; sd= 0.252; skew= -0.019 area= 80.8scm1; slope= 10.9f1/m1; length-	14.8ai	1955-88 1953	7,720 10	12,400	15,700 13	19,800 16	22,700 18	25,900 21	28,500 23	32,300 27	
314	note: *	Beaver Run near McCall Crock, MS mean= 2.607; sd= 0.298; skew= -0.193 area= 2.65sqmi; slope= 37.0ft/mi; length=		1955-77	471 14	813 14	1,070 16	1,430 19	1,680 21	1,970 24	2,180 27	2,500 30	
315	note: *	Homochitto River near Bude, MS mean- 4.506; sd= 0.183; skew= 0.263 area- 407 sqmi; slopc= 5.8ft/mi; length-	39.7mi	1942-50 1972-74 1961,65	30,100 12	43,500 13	52,900 15	65,400 19	74,300 22	85,900 24	95,100 27	109,000	
316	7292500 note: ag region 2	Homochitto River at Rosetta, MS mean- 4.698; sd= 0.326; skew0.447 area- 787 sqmi; slope= 5.0ft/mi; length-	64.4mi	1952-88	52,700 14	94,800 13	125,000 14	164,000 18	194,000 22	223,000 27	252,000 32	290,000 40	
317	note: aed	Second Crock at Sibloy, MS g mean= 3.991; sd= 0.264; skew= -0.176 arca= 55.3sqmi; slope= 9.2ft/mi; length=	23.3mi	1952-59	9,980 24	16,400 24	21,100 28	27,400 36					
318	note: *	Observer's Draw near Doloroso, MS mean- 2.290; sd= 0.185; skcw= 0.232 area- 0.22sqmi; slope= 153 ft/mi; length-	1.1mi	1954-77	194 9	280 11	338 13	412 16	460 19	514 22	550 24	613 28	
319	note: af	Homochitto River near Doloroso, MS mean= 4.710; sd= 0.266; skow= -0.255 area= 1,140 sqmi; slope= 4.0ft/mi; length=	85.3mi	1940-58 1972-78 1938,61,65 1969,83	52,600 12	86,500 12	110,000	142,000 17	166,000 21	190,000 26	215,000 30	248,000 37	
320	note: *	Buffalo River near Woodville, MS mean= 4.382; sd= 0.258; skew= -0.572 area= 180 sqmi; slope= 7.5fl/mi; length=		1942-88	24,200 9	38,200 8	46,700 9	55,600 11	60,500 13	65,900 16	70,000 19	75,800 22	
321	note: *	West Fork Thompson Creek near Wakefield, LA mean= 3.792; sd- 0.293; skew=-0.276 area= 35.3sqmi; slope=11.8ft/mi; length=]5.lmi	1950-70	5,860 15	9,€60 14	12,100 15	14,600 18	15,900 21	17,600 24	18,800 26	20,600 30	
322	note: *	Moores Branch near Woodville, MS mean= 2.323; sd= 0.203; skew0.371 area= 0.21sqmi; slope- 49.Cft/mi; length=	0.7mi	1955-88	210 9	300 8	353 9	411 11	443 13	475 16	498 18	534 21	
323	note: *	<pre>Tangipahoa River tributary near McComb, MS moan= 2.735; sd= 0.225; skew= -0.244 area= 2.82sqmi; slope= 26.3ft/mi; length=</pre>	3.0mi	1966-84	579 12	881 12	1,090 13	1,370 16	1,580 18	1,810 21	2,000 24	2,280 27	
324	noto: *	Little Tangipahoa River at Magnolia, MS mean= 3.437; sd= 0.405; skew= -0.347 area= 39.8sqmi; slope= 9.7ft/mi; length=	14.6mi	1960-73	3,330 22	6,130 21	8,130 22	10,600 24	12,000 27	13,900 29	15,200 32	17,200 35	

Map Station

no.

top line-- Peak T-year flood magnitude (cubic feet per second) bottom line-- Standard error of T-year flood estimate (percent)

2-year 5-year 10-year 25-year 50-year 100-year 200-year 500-year

325 7375500 Tangipahoa River at Robert, LA note: * mean= 4.138; sd= 0.334; skew= -0.139 1939-88 15,300 29,100 40,600 58,200 72,600 89,500 105,000 127,000 note: * mean= 4.138; sd= 0.334; skew= -0.139 region 2 area= 646 sqm1; slope= 6.0ft/m1; length= 66.0m1 11 11 13 16 18 21 23

326 7375800 Tickfaw River at Liverpool, LA 1956-88 4,210 9,080 13,400 19,600 23.900 28,900 32,700 38,100 note: * mean= 3.556; sd= 0.413; skew= 0.077
region 2 area= 89.7sqm1; slope= 8.7ft/mi; length= 16.9mi 24 30 327 7376665 Stock Pond Draw near Liberty, MS 1965-77 217 323 390 470 574 517 609 674 note: * mean= 2,336; sd= 0.223; skew= -0.018 region 2 area= 0.38sqmi; slope= 56.3ft/mi; length= 17 20 23 1955 1.2m1

Table 1.--Flood quantiles, standard error of estimate, and other information for gaging stations--Continued

328 7376720 Tanyard Creek at Liberty, MS
note: c mean= 2.980; sd= 0.678; skew= -0.228
region 2 area= 9.92sgmi; slope= 14.7ft/mi; length= 1953-70 1,330 2,710 3,600 4,530 4,980 5,640 6,010 6,690 27 25 26 28 29 31 1973 7.0mi 329 7376760 CRS Draw near Liberty, MS mean= 2.694; sd= 0.127; skew= -0.105 1965-84 492 626 709 809 879 952 1,020 1,110 note: * mean= 2.694; sd= 0.127; skew= -0.105 region 2 area= 0.80sqm1; slope= 48.5ft/m1; length= 14 1.3m1 1955

330 7377000 Amite River near Darlington, LA 1949-88 21,500 41,900 58,000 80,600 97,200 116,000 132,000 154,000 note: * mean= 4.281; sd- 0.375; skew= -0.346 14 13 14 17 20 22 25 29 region 2 area= 580 sqm1; slope= 6.4ft/m1; length= 41.6mi

- The station was used in the computation of regional flood-frequency equations.
- a The station is affected by regulation or channelization. The estimates may have more uncertainty than is indicated by the standard error of estimate. If station is affected by channelization, the slope and length may not be fully representative of existing conditions.
- b The station is affected by regulation, channelization, or urbanization. Record collected prior to basin changes was used to contribute to the regional analysis. The flood magnitudes, if shown, are estimates for current conditions.
- c The station was not used in regional analyses because of regionally uncharacteristic flood frequency or other sample problems. Flood magnitudes are weighted estimates.
- \ensuremath{d} . The drainage basin is significantly urbanized. The flood magnitudes are unweighted stations estimates,
- e The period of record is insufficient for station estimates of large recurrence
- f The logarithmic mean and standard deviation of the station record were adjusted by correlation with long-term records on the same stream using procedures described in Bulletin 17B (Interagency Advisory Committee on Water Data, 1982).

- 9 The flood magnitudes are unweighted station estimates.
- $\ensuremath{\mathsf{h}}$. The drainage basin characteristics are outside the limitations of the regional equations,
- The post-regulation or post-channelization period of record is insufficient for flood-frequency analysis. The statistics of logarithms of annual peak flow are for natural conditions.
- j The unweighted flood-flow estimates are for existing project conditions and represent combined flow in the natural and regulated channels. The flood-flow values were obtained from the U.S. Army Corps of Engineers, Mobile District (written commun., April 1990). The statistics of logarithms of annual peak flow are for natural conditions.
- k The logarithmic mean of annual peak flow is for regulated conditions. The flood magnitudes and the logarithmic standard deviation and skew of annual peak flow are not presented in this report. For additional information, contact the U.S. Army Corps of Engineers, Vicksburg District.
- 1 Peak T-year flood magnitude is in thousands of cubic feet per second.

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APPENDIX

REGIONAL SKEW COEFFICIENTS